



# Article Anomalous Solute Transport Using Adsorption Effects and the Degradation of Solute

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Abstract: In this work, anomalous solute transport using adsorption effects and the decomposition of solute was studied. During the filtration of inhomogeneous liquids, a number of new phenomena arise, and this is very important for understanding the mechanisms of the filtration process. Recently, issues of mathematical modeling of substance transfer processes have been intensively discussed. Modeling approaches are based on the law of matter balance in a certain control volume using additional phenomenological relationships. The process of anomalous solute transport in a porous medium was modeled by differential equations with a fractional derivative. A new mobile—immobile model is proposed to describe anomalous solute transport with a scale-dependent dispersion in inhomogeneous porous media. The profiles of changes in the concentrations of suspended particles in the macropore and micropore were determined. The influence of the order of the derivative with respect to the coordinate and time, i.e., the fractal dimension of the medium, was estimated based on the characteristics of the solute transport in both zones. The hydrodynamic dispersion was set through various relations: constant, linear, and exponential. Based on the numerical results, the concentration fields were determined for different values of the initial data and different relations of hydrodynamic dispersion.

**Keywords:** anomalous transfer; solute adsorption; hydrodynamic dispersion; fractional derivative; solute diffusion; mass transfer; porous medium; filtration

### 1. Introduction

An important and complex issue in the study of anomalous solute transport is scaledependent dispersion [1–4]. The essence of scale-dependent dispersion is that the dispersion power or dispersion coefficient changes with distance or time when using the convection—dispersion equation to describe the process of anomalous solute transport in porous media [3,5–11].

Most analytical studies related to scale-dependent dispersion are based on the convection—dispersion equation. However, in recent years many researchers have often questioned the use of the convection—dispersion equation, since it cannot adequately explain anomalous transport in inhomogeneous porous media, and alternative models have been proposed. The mobile—immobile model differs from the convection—dispersion model in that it consists of the presence of a stagnant region in a porous medium and the exchange of dissolved substances between mobile and stagnant regions, which explains the early entry and long transport time of solutes. The solute flux in the mobile—immobile model is proportional to the concentration and the difference between these two areas. More detailed information related to the analysis of the mobile—immobile model can be found in [12]. It was shown that the mobile—immobile model can better describe the transport of dissolved substances in both homogeneous and inhomogeneous porous media than the convection—dispersion equation [13–19].



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**Copyright:** © 2023 by the authors. Licensee MDPI, Basel, Switzerland. This article is an open access article distributed under the terms and conditions of the Creative Commons Attribution (CC BY) license (https:// creativecommons.org/licenses/by/ 4.0/). limited when the exchange time scale is equal to or exceeds the characteristic advection time scale through the medium [20,21]. Some studies describe how rate-limited transport processes may be better suited to describing solute transport in a macro dispersion experiment than a macro dispersion model [20,22]. Another widely used velocity-limited mass transfer model is the physical diffusion model, in which the diffusion into and out of a fixed zone is described by Fick's law [23,24]. However, the diffusion model is limited to structured soils with geometrically well-defined aggregates [12]. In addition, different expressions for the first order mass transfer rate can be obtained for a diffusion model with some idealized aggregate shapes [23].

It should be noted that the mobile—immobile model considered above uses a lumped unified velocity to describe a complex mass transfer process. However, for natural soils or complex and heterogeneous aquifers characterized by microscale variations in the properties of the porous media, the rates and types of mass transfer differ greatly [21,24]. The single-speed mobile—immobile model cannot accurately describe the nature of long-term transport caused by multiple and simultaneous mass transfer processes [19,25]. A multivelocity mobile—immobile model was developed using a distribution of mass transfer coefficients in combination with the convection—dispersive model of solute transport [24]. The continuum of velocities in a memory function and the resulting equations of the mobile—immobile model with several velocities were defined as a pair of coupled deterministic partial differential equations in continuous time and space with one derivative with respect to time [21]. The single-velocity mobile—immobile model and diffusion models for various geometries, as special cases of the multi-velocity mobile—immobile model, were studied [24–27].

The mobile and immobile model separates the liquid phase of a porous medium into a mobile and a stationary region. It is assumed that the convective—dispersive transport is limited to the region of the mobile fluid and that the exchange of the solutes between these two regions of the fluid can be described as a first order process. The solid phase of the porous medium is also divided into two adsorption centers that are instantly balanced with the mobile and immobile liquid regions, respectively. The adsorption of a solute by a solid phase is described by a linear isotherm and the degradation of a solute in both liquid and solid phases is considered a first order process. To maintain the generality of the model, different degradation coefficients were taken into account in the mobile and stationary liquid regions as well as in the mobile and stationary adsorbed phases [28]. The mobile—immobile (M&IM) model was developed on the basis of the two-region or two-site model, which divides heterogeneous porous media into mobile and immobile regions [13,29]. The solute transport and kinetic equations were considered to analyze reactive solute transport and the analytical method was used to describe the results [30].

In this study, the process of anomalous solute transport in a porous medium is modeled by differential equations with fractional derivatives. A new "mobile—immobile" model is proposed to describe the anomalous solute transport with a scale-dependent dispersion in inhomogeneous porous media.

The mobile region is the zone with the moving fluid and the immobile region is the zone with the stationary fluid. The mobile region governs flow transport processes, i.e., advection and the dispersion process, and the stagnant or immobile region accounts for the first-order lumped mass transfer between mobile and immobile regions. The profiles included in the concentrations of suspended particles in the macropore and micropore were determined. The influence of the order of the derivatives with respect to the coordinate and time on the characteristics of the solute transport in both zones was estimated. In the zone with the immobile liquid, the transfer process is described by a kinetic equation accounting for adsorption, where, unlike in other well-known works, the anomalous process is also taken into account. In the zone with the mobile liquid, a convective—diffusion equation is used, taking into account the anomalous diffusion process. Further, we use the "mobile"

and "immobile" zones, which means the zones with mobile and immobile fluid are studied and presented in the results as graphs.

#### 2. Materials and Methods

## 2.1. Formulation of the Problem

The environment consists of two zones, one is mobile, i.e., porous medium, where the liquid is mobile, and the other is immobile, where the liquid is immobile. Diffusion solute transport occurs in both zones (See Figure 1).



Figure 1. Scheme of the solute transport in two-zone medium.

The model of anomalous solute transport is written as [29–31]:

$$(\theta_m + f\rho_b k_d) \frac{\partial c_m}{\partial t} = \theta_m \frac{\partial}{\partial x} \left[ D_m(x) \frac{\partial^\beta c_m}{\partial x^\beta} \right] - v_m \theta_m \frac{\partial c_m}{\partial x} - \omega(c_m - c_{im})$$

$$- (\theta_m \mu_{lm} + f\rho_b k_d \mu_{sm}) c_m,$$

$$(1)$$

$$[\theta_{im} + (1-f)\rho_b k_d] \frac{\partial^{\alpha} c_{im}}{\partial t^{\alpha}} = \omega(c_m - c_{im}) - [\theta_{im}\mu_{lim} + (l-f)\rho_b k_d\mu_{sim}]c_{im}$$
(2)

where  $\theta_m$ ,  $\theta_{im}$  are porosity coefficients,  $c_m$  and  $c_{im}$  are the concentrations of dissolved substances in the mobile and immobile zones, respectively,  $v_m$  is the fluid velocity,  $\omega$  is the first-order mass transfer coefficient, f and l - f represent the proportions of adsorption centers that are instantly balanced with the areas of mobile and immobile liquid, respectively,  $\rho_b$  is the density of the porous medium,  $k_d$  is the distribution coefficient of the linear adsorption process,  $\mu_{lm}$  and  $\mu_{lim}$  are the degradation coefficients of the dissolved substance in the mobile and immobile zones, respectively,  $\mu_{sm}$  and  $\mu_{sim}$  are the degradation coefficients of the adsorbed substance in the mobile and immobile zones, respectively, x is the spatial coordinate, t is the time, and  $D_m(x)$  is the coefficient of hydrodynamic dispersion in the moving zone expressed as [32]:

$$D_m(x) = \phi(x)v_m + D_0 \tag{3}$$

 $\phi(x)$  is the dispersion and  $D_0$  is the diffusion coefficient.

The parameter values are taken as [30] and the orders of the derivatives are considered as:  $0 < \alpha \le 1, 0 < \beta \le 1$ . In contrast to [30], here we take other dimensions as  $[D_m(x)] = m^{\beta+1}/s$ .  $[\theta_{im} + (1-f)\rho_b k_d] = s^{\alpha-1}$  represents fractal dimensions of the parameters.

Using  $R_m = (\theta_m + f\rho_b k_d)$ ,  $R_{im} = (\theta_{im} + (1 - f)\rho_b k_d)$ ,  $A_1 = (\theta_m \mu_{lm} + f\rho_b k_d \mu_{sm})$ , and  $A_2 = (\theta_{im} \mu_{lim} + (1 - f)\rho_b k_d \mu_{sim})$ , Equations (1) and (2) can be transformed into a simpler form as follows:

$$R_m \frac{\partial c_m}{\partial t} = \theta_m \frac{\partial}{\partial x} \left[ D_m(x) \frac{\partial^\beta c_m}{\partial x^\beta} \right] - v_m \theta_m \frac{\partial c_m}{\partial x} - \omega(c_m - c_{im}) - A_1 c_m, \tag{4}$$

$$R_{im}\frac{\partial^{\alpha}c_{im}}{\partial t^{\alpha}} = \omega(c_m - c_{im}) - A_2 c_{im},$$
(5)

where  $R_m$ ,  $R_{im}$  are the retardation factors in the moving and stationary zones, respectively.

For mobile and immobile models with scale-dependent dispersion, the dispersion is not a constant, but a function of distance. Here, as in previous experimental and theoretical studies [33], we use linear and exponential dispersion functions. Distance-dependent linear variance increases without limit with distance, while distance-dependent exponential variance initially increases with distance and eventually approaches an asymptotic value. The formula for linear distance-dependent dispersion is:

$$p(x) = kx, \tag{6}$$

where *k* is the slope of the ratio of dispersion to distance (dimensionless). The exponential dependence of dispersion on distance is expressed as:

$$\phi(x) = a \Big( 1 - e^{-bx} \Big), \tag{7}$$

where *a* is the asymptotic value at infinity and *a*, *b* are positive constants.

Consider the problem of a fluid with a substance concentration,  $c_0$ , movable zone from x = 0. Initially, both the zones are filled with pure (without substance) liquid.

The initial and boundary conditions for this setting are:

$$c_m(0, x) = 0, c_{im}(0, x) = 0,$$
 (8)

$$c_m(t, 0) = c_0, c_m(t, \infty) = 0.$$
 (9)

# 2.2. Solution Procedure

For the numerical solution of Equations (3)–(9), we use the finite difference method [34]. In the domain,  $\Omega = \{0 \le x \le \infty, 0 \le t \le T\}$  introduces a uniform grid method,  $\omega_{h\tau} = \{(x_i, t_j), x_i = ih, i = \overline{0, N}, h = L/N, t_j = j\tau, j = \overline{0, M}, \tau = T/M\}$ , where *h* is the grid step size at coordinate  $x, \tau$  is the grid step at time *t*, and *L* is the characteristic length of the porous medium, chosen so that the concentration field does not reach x = L within the considered time range.

To approximate the fractional time derivatives, we use the schemes given in [35–37]. The difference approximation of the kinetic equation (Equation (5)) is written as:

$$\frac{R_{im}}{\Gamma(2-\alpha)\tau^{\alpha}} \left( \sum_{l=0}^{j-1} \left( (c_{im})_{i}^{l+1} - (c_{im})_{i}^{l} \right) \left( (j-l+1)^{1-\alpha} - (j-l)^{1-\alpha} \right) + \left( (c_{im})_{i}^{j+1} - (c_{im})_{i}^{j} \right) \right) = \left( \omega((c_{m})_{i}^{j} - (c_{im})_{i}^{j}) - A_{2}(c_{im})_{i}^{j} \right).$$
(10)

For the three cases of dispersion coefficients (constant, linear, asymptotic exponential), Equation (4) is approximated in the following forms.

1. Constant case

$$D_m(x) = \phi(x)v_m + D_0, \ \phi(x) = 0, \ D_m(x) = D_0$$

$$R_m \frac{\partial c_m}{\partial t} = \theta_m D_0 \frac{\partial^{\beta+1} c_m}{\partial x^{\beta+1}} - v_m \theta_m \frac{\partial c_m}{\partial x} - \omega (c_m - c_{im}) - A_1 c_m,$$

$$R_{m} \frac{(c_{m})_{i}^{j+1} - (c_{m})_{i}^{j}}{\tau} = \frac{\theta_{m} D_{0}}{\Gamma(3 - (\beta + 1))h^{\beta + 1}} \sum_{l=0}^{i-1} \left( (c_{m})_{i-(l+1)}^{j} - 2(c_{m})_{i-l}^{j} + (c_{m})_{i-(l-1)}^{j} \right) \\ \times \left( (l+1)^{2 - (\beta + 1)} - (l)^{2 - (\beta + 1)} \right) - v_{m} \theta_{m} \frac{(c_{m})_{i+1}^{j} - (c_{m})_{i-1}^{j}}{2h} - \omega((c_{m})_{i}^{j} - (c_{im})_{i}^{j}) - A_{1}(c_{m})_{i}^{j},$$

$$(11)$$

2. Linear case

$$D_m(x) = \phi(x)v_m + D_0, \ \phi(x) = kx, \ L_1 = kv_m, \ D_m(x) = kxv_m + D_0$$

$$R_m \frac{\partial c_m}{\partial t} = \theta_m \frac{\partial}{\partial x} \left[ (kxv_m + D_0) \frac{\partial^\beta c_m}{\partial x^\beta} \right] - v_m \theta_m \frac{\partial c_m}{\partial x} - \omega (c_m - c_{im}) - A_1 c_m,$$

$$R_m \frac{\partial c_m}{\partial t} = \theta_m \left[ k v_m \frac{\partial^\beta c_m}{\partial x^\beta} + (k x v_m + D_0) \frac{\partial^{\beta+1} c_m}{\partial x^{\beta+1}} \right] - (v_m \theta_m) \frac{\partial c_m}{\partial x} - \omega (c_m - c_{im}) - A_1 c_m,$$

$$R_{m} \frac{(c_{m})_{i}^{j+1} - (c_{m})_{i}^{j}}{\tau} = \theta_{m} \left( k v_{m} \frac{(c_{m})_{i+1}^{j} - \beta \cdot (c_{m})_{i}^{j}}{\Gamma(2-\beta)h^{\beta}} \right) + \theta_{m} (kihv_{m} + D_{0}) \\ \times \frac{\left(\sum_{l=0}^{i-1} \left( (c_{m})_{i-(l+1)}^{j} - 2(c_{m})_{i-l}^{j} + (c_{m})_{i-(l-1)}^{j} \right) \left( (l+1)^{2-(\beta+1)} - (l)^{2-(\beta+1)} \right) \right)}{\Gamma(3-(\beta+1))h^{\beta+1}} - v_{m}\theta_{m} \frac{(c_{m})_{i+1}^{j} - (c_{m})_{i-1}^{j}}{2h}}{-\omega((c_{m})_{i}^{j} - (c_{im})_{i}^{j}) - A_{1}(c_{m})_{i}^{j}}.$$
(12)

# 3. Exponential case

$$D_m(x) = \phi(x)v_m + D_0, \phi(x) = a(1 - e^{-bx}), D_m(x) = a(1 - e^{-bx})v_m + D_0.$$

$$R_{im}\frac{\partial c_m}{\partial t} = \theta_m \frac{\partial}{\partial x} \left[ (av_m(1-e^{-bx})+D_0)\frac{\partial^\beta c_m}{\partial x^\beta} \right] - v_m \theta_m \frac{\partial c_m}{\partial x} - \omega(c_m-c_{im}) - A_1 c,$$

$$R_{im}\frac{\partial c_m}{\partial t} = \theta_m \left( (av_m - av_m e^{-bx} + D_0) \frac{\partial^{\beta+1} c_m}{\partial x^{\beta+1}} + (abv_m e^{-bx}) \frac{\partial^{\beta} c_m}{\partial x^{\beta}} \right) - v_m \theta_m \frac{\partial c_m}{\partial x} - \omega (c_m - c_{im}) - A_1 c,$$

$$R_{m} \frac{(c_{m})_{i}^{j+1} - (c_{m})_{i}^{j}}{\tau} = \frac{\theta_{m}(av_{m} + av_{m}e^{-ih} + D_{0})}{\Gamma(3 - (\beta + 1))h^{\beta + 1}} \sum_{l=0}^{i-1} \left( (c_{m})_{i-(l+1)}^{j} - 2(c_{m})_{i-l}^{j} + (c_{m})_{i-(l-1)}^{j} \right) \times \left( (l+1)^{2 - (\beta + 1)} - (l)^{2 - (\beta + 1)} \right) + \theta_{m} \left( (abv_{m}e^{-bih}) \frac{(c_{m})_{i+1}^{j} - \beta(c_{m})_{i}^{j}}{\Gamma(2 - \beta)h^{\beta}} \right) - v_{m}\theta_{m} \frac{(c_{m})_{i+1}^{j} - (c_{m})_{i-1}^{j}}{2h}$$

$$-\omega((c_{m})_{i}^{j} - (c_{im})_{i}^{j}) - A_{1}(c_{m})_{i}^{j}.$$
(13)

The initial and boundary conditions are approximated as follows:

$$(c_m)_i^0 = 0 \ (c_{im})_i^0 = 0,$$
 (14)

$$(c_m)_0^j = c_0, \quad (c_m)_N^j = 0.$$
 (15)

where *N* is a sufficiently large number for which the equation  $c_N^j = 0$  is approximately satisfied.

## 3. Results and Discussion

The classical case of the newly proposed model was compared with the results obtained from previous work [29] for the constant case and is shown in Figure 2. Here,  $\alpha = 1$  and  $\beta = 1$  are fixed in the fractional derivative. It can be seen from the figure that the new model in reduced cases coincides exactly with the solution of the problem in the classical case.



**Figure 2.** Concentration profile for solute transport at time T = 3600.

The following values of the initial parameters were used to analyze the new results [29,30]:  $\theta_m = 0.4$ ,  $\theta_{im} = 0.1$ ,  $\rho_b = 1000$ ,  $k_d = 10^{-6}$ ,  $\mu_{lm} = 10^{-3}$ ,  $\mu_{sm} = 10^{-4}$ ,  $\mu_{lim} = 10^{-6}$ ,  $\mu_{sim} = 10^{-8}$ ,  $v_m = 10^{-4}$ ,  $\omega = 10^{-5}$ ,  $D_0 = 10^{-5}$ , T = 3600, f = 0.7, k = 0.5, a = 1, b = 0.5. These values were used to solve Equations (10)–(13). Three different hydrodynamic dispersion coefficients, namely, constant, linear, and exponential coefficients, are discussed for the concentration profiles of mobile and immobile zones, and the results are presented in the figures.

Figure 3a–d shows the changes in substance concentrations for various hydrodynamic dispersion coefficients (constant, linear, and asymptotic) and various  $\alpha$  and  $\beta$  values. With a decrease in the order of the derivative for  $\beta \ge 1$  in the diffusion term of the mass transfer equation, a wider distribution of the concentration profile in the mobile and immobile zones is observed.



**Figure 3.** Concentration profiles (a):  $c_m$  at  $\alpha = 1$ ,  $\beta = 1$ ; (b):  $c_m$  at  $\alpha = 1$ ,  $\beta = 0.9$ ; (c)  $c_{im}$  at  $\alpha = 1$ ,  $\beta = 1$  (d)  $c_{im}$  at  $\alpha = 1$ ,  $\beta = 0.9$ , t = 3600. Constant: ...., Linear: ..., Exponential:

1. Constant case

Figures 4–7 depict a case in which the hydrodynamic dispersion coefficient is constant. Figure 4 shows the variation in concentration values in the mobile and immobile zones

for different values of  $\beta$ , in which of  $\alpha = 1$  and t = 3600 are fixed values. In this case, the distribution of concentration profiles in both zones becomes wider. Similarly, Figure 5 shows the variation in the concentration value in the mobile and immobile zones for different values of  $\beta$ , in which  $\alpha = 0.8$  and t = 3600 are fixed values. The pattern of the variation is the same as shown in Figure 4.



**Figure 4.** Concentration profiles (a):  $c_m$ , (b):  $c_{im}$  at  $\alpha = 1$ , t = 3600.  $\beta = 0.8$ : ....,  $\beta = 0.9$ :



**Figure 5.** Concentration profiles (**a**):  $c_m$ , (**b**):  $c_{im}$  at  $\alpha = 0.8$ , t = 3600.  $\beta = 0.8$ : .....,  $\beta = 0.9$ :



**Figure 6.** Concentration profiles (a):  $c_m$ , (b):  $c_{im}$  at  $\beta = 1$ , t = 3600.  $\alpha = 0.8$ : ....,  $\alpha = 0.9$ :

Figures 6 and 7 show the values of the mobile and immobile zones for  $\beta = 1$  and  $\beta = 0.8$ , respectively, with different values of  $\alpha$  ( $\alpha = 0.8$ , 0.9, and 1.0). In Figures 6a and 7a, the differences in the values of the distribution of concentration profiles are very small (maximum difference is  $6.80973 \times 10^{-5}$ ). It can be seen from the figures that the concentration profiles are more widely distributed in the mobile zone, but less widely distributed in the immobile zone.



**Figure 7.** Concentration profiles (**a**):  $c_m$ , (**b**):  $c_{im}$  at  $\beta = 0.8$ , t = 3600.  $\alpha = 0.8$ : ....,  $\alpha = 0.9$ : ...,  $\alpha = 0.9$ : ...,  $\alpha = 1.0$ : ...,  $\alpha = 0.8$ : ...,

# 2. Linear case

Figures 8–11 show the results for a linearly varying hydrodynamic dispersion coefficient.



**Figure 8.** Concentration profiles (a):  $c_m$ , (b):  $c_{im}$  at  $\alpha = 1$ , t = 3600.  $\beta = 0.8$ : ....,  $\beta = 0.9$ :



**Figure 9.** Concentration profiles (a):  $c_m$ , (b):  $c_{im}$  at  $\alpha = 0.8$ , t = 3600.  $\beta = 0.8$ : ....,  $\beta = 0.9$ :



**Figure 10.** Concentration profiles (**a**):  $c_m$ , (**b**):  $c_{im}$  at  $\beta = 1$ , t = 3600.  $\alpha = 0.8$ : ....,  $\alpha = 0.9$ :



**Figure 11.** Concentration profiles (a):  $c_m$ , (b):  $c_{im}$  at  $\beta = 0.8$ , t = 3600.  $\alpha = 0.8$ : ....,  $\alpha = 0.9$ :

Figures 8 and 9 depict the change in concentration profiles for different values of  $\beta$ , in which  $\alpha = 1$  and  $\alpha = 0.8$ , respectively. The concentration profiles decrease with respect to the length parameter, which is higher for higher values of  $\beta$ .

Figures 10 and 11 show the concentration profile for the time derivatives  $\alpha = 0.8$ , 0.9 and 1.0, fixing  $\beta = 1$  and  $\beta = 0.8$ , respectively. These figures show that the concentration profiles are more widely distributed in the mobile zone, while in the stationary zone, on the contrary, they decrease.

# 3. Exponential case

The results for the exponential hydrodynamic dispersion coefficient are shown in Figures 12–15. With a decrease in the value of  $\beta$ , a wider distribution of concentration profiles in both zones is observed (Figures 12 and 13) for  $\beta = 1$  and  $\beta = 0.8$ . Decreasing the order of the time derivative,  $\alpha$ , less than 1 in the mass transfer equation, the concentration profiles spread more widely in the mobile zone, but spread less in the stationary zone (Figures 14 and 15).



**Figure 12.** Concentration profiles (a):  $c_m$ , (b):  $c_{im}$  at  $\alpha = 1$ , t = 3600.  $\beta = 0.8$ : ....,  $\beta = 0.9$ :



**Figure 13.** Concentration profiles (a):  $c_m$ , (b):  $c_{im}$  at  $\alpha = 0.8$ , t = 3600 s.  $\beta = 0.8$ : ....,  $\beta = 0.9$ :



**Figure 14.** Concentration profiles (a):  $c_m$ , (b):  $c_{im}$  at  $\beta = 1$ , t = 3600 s.  $\alpha = 0.8$ : ....,  $\alpha = 0.9$ :



**Figure 15.** Concentration profiles (**a**):  $c_m$ , (**b**):  $c_{im}$  at  $\beta = 0.8$ , t = 3600 s.  $\alpha = 0.8$ : ....,  $\alpha = 0.9$ : ...,  $\alpha = 0.9$ : ...,  $\alpha = 1.0$ : ...

The results of calculations related to the change in the adsorption values are shown in Figures 16–18. As can be seen from Figure 16, a decrease in  $\beta$  from 1, as in the previous cases, leads to an increase in diffusion effects in both zones. Comparison of these results with the corresponding ones for  $k_d = 10^{-4}$ ,  $k_d = 10^{-3}$ , and  $k_d = 10^{-2}$  shows that adsorption leads to a general slowdown in the process of distribution of the substance. The change in the concentration profiles at different adsorption values is shown in Figures 17 and 18. With an increase in the adsorption coefficient, a lagging distribution of concentration profiles is observed.



Figure 16. Cont.





Figure 17. Cont.





**Figure 18.** Concentration profiles  $c_m$  (**a**-**c**),  $c_{im}$ (**d**-**f**) at  $\alpha = 1$ ,  $\beta = 1$ , t = 3600. (**a**,**d**):  $k_d = 10^{-4}$ , (**b**,**e**):  $k_d = 10^{-3}$ , (**c**,**f**):  $k_d = 10^{-2}$ , Constant: ..., Linear: ..., Exponential:

### 4. Conclusions

Solute transport in a two-zone porous medium with mobile and immobile fluid was analyzed with inclusion of adsorption. Caputo's definition was used to determine the numerical solution of the fractional differential equation. The anomalous transport was characterized by the order of the derivative in the diffusion terms of the transport equations in the macropore and the micropore. In this study, reducing the order of the derivative in the diffusion terms of the transport equations in both the zones led to fast diffusion. Reducing the order of the derivative for  $\alpha$  below one leads to slow diffusion in the micropore. In this sense, the presence of a zone with a fluid and adsorption affects the transport characteristics in a similar way. It was shown that in the case of equilibrium adsorption, an increase in the adsorption coefficient leads to a general slowdown in the process of spreading a substance in a medium.

Decreasing the diffusion term from the derivative has been shown to accelerate the diffusion process. On the contrary, decreasing the order of the derivative in the time-dependent change in concentration for  $0 < \alpha \leq 1$  leads to a decrease in the diffusion process (slow diffusion). It was observed that decreasing the order of the derivative of the diffusion term in the zone  $0 < \beta \leq 1$  affects only the migration characteristics in the zone, while decreasing the derivative order of the diffusion term in the zone  $0 < \beta \leq 1$  affects the migration characteristics in both zones.

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#### Nomenclature

- *a* constant in exponential distance-dependent dispersivity (*m*)
- *b* constant in exponential distance-dependent dispersivity (1/m)
- $c_m$  solute concentration in mobile region
- *c*<sub>*im*</sub> solute concentration in immobile region
- $c_0$  constant source concentration
- $D_m$  dispersion coefficient in mobile region  $(m^{\beta+1}/s)$
- $D_0$  molecular diffusion coefficient  $(m^{\bar{\beta}+1}/s)$
- *f* fraction of adsorption sites equilibrating instantaneously with mobile liquid region
- *k* constant in linear distance-dependent dispersivity
- $k_d$  distribution coefficient for linear sorption
- t time (s)
- $v_m$  mobile pore-water velocity
- *x* Distance
- $\theta_m$  water content in mobile region
- $\theta_{im}$  water content in immobile region
- $\phi$  dispersivity (*m*)
- $\omega$  mass transfer coefficient (1/s)
- $\rho_b$  bulk density of porous medium
- $\mu_{lm}$  first-order degradation coefficient in mobile adsorbed solid phase (1/s)
- $\mu_{lim}$  first-order degradation coefficient in immobile liquid region (1/s)
- $\mu_{sm}$  first-order degradation coefficient in mobile adsorbed solid phase (1/s)
- $\mu_{sim}$  first-order degradation coefficient in immobile adsorbed solid phase (1/s)

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