



Article A Novel Wave Energy Equivalence Based Lumping Block Method for Efficiently Predicting the Fatigue Damage of Mooring Lines

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Abstract: The lumping block equivalent method (LBEM) is widely used to reduce the computational effort in the fatigue damage assessment of offshore structures, and the wave parameters of the representative sea states (RSSs) resulting from LBEM are of vital importance for the accurate prediction of offshore structures' fatigue damage. In this study, a novel wave energy equivalence (WEE)-based LBEM is proposed to determine the wave parameters of the RSS accurately. The novelty of the proposed method is that a compact relationship between the input wave energy component and mooring lines' fatigue damage is derived, and the modified statistical relationships between the wave parameters and spectral moments are provided by incorporating the effects of the peak enhancement factor of the input wave spectrum, the number of original sea states (OSSs) and the equivalence bandwidth of the OSSs. Based on the compact relationship, the wave energy component of the RSS can be determined from the wave energy component of the OSSs for each wave frequency from the viewpoint of the fatigue damage equivalence criterion. The wave energy distribution of the RSS can be accurately characterized with the wave energy distribution of the OSSs, and the spectral moments of the RSS can be calculated by its energy distribution directly, without any approximation. Moreover, the wave parameters of the RSS can be determined from the modified statistical relationships easily. The effectiveness of the proposed WEE LBEM is numerically investigated with a moored semisubmersible platform. Results show that the proposed WEE LBEM is robust, efficient and accurate within engineering expectations, and it outperforms the conventional LBEMs both in accuracy and robustness.

Keywords: floating offshore structure; fatigue damage assessment; lumping block equivalent method; wave energy equivalence; mooring system

1. Introduction

A floating offshore structure generally composed of large-scaled floaters and slender mooring lines is the infrastructure used for the exploitation of natural resources in deep and ultra-deep water [1,2]. When the floating offshore structure is in operational conditions, the stochastic wave acting on the structure can be considered as the cyclic hydrodynamic load, which can lead the mooring lines to experience fatigue failure, even if the mooring line's tension is much smaller than its minimum breaking load [3,4]. Therefore, the mooring line's fatigue damage is one of governing factors for the design of floating offshore structures [5–7].

Generally, through an integration account of efficiency and safety requirements, the classical frequency- and time-domain fatigue assessment methods are the most commonly used methods in the design stage of floating offshore structures [8,9]. However, due to the nonlinearities inherent in the system, the dynamic responses of the floating offshore structure are non-Gaussian processes, and the classical frequency-domain fatigue assessment



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Copyright: © 2023 by the authors. Licensee MDPI, Basel, Switzerland. This article is an open access article distributed under the terms and conditions of the Creative Commons Attribution (CC BY) license (https:// creativecommons.org/licenses/by/ 4.0/). method, which is based on the Rayleigh distribution, therefore becomes inapplicable [3,10]. The time-domain fatigue assessment method can consider the nonlinearities of the system and the hydrodynamic loads accurately via a coupled dynamic analysis model, and the dynamic response can be converted into a series of response ranges and cycle numbers with the aid of the rain-flow counting algorithm [11]. As a result, it can be used as the benchmark for other methods [12,13]. However, the time-domain method has a remarkable computational cost. On the one hand, the short-term variability contributed by the stochastic wave necessitates multiple realizations to acquire statistical convergence. On the other hand, the simulation duration should be long enough to obtain sufficient low-frequency cycles to accurately estimate the mooring line's fatigue damage [14–16].

To reduce the computational cost, the lumping block equivalent method (LBEM), utilizing one representative sea state (RSS) to replace a group of original sea states (OSSs) to estimate the offshore structure's fatigue damage, is widely adopted in engineering practice [5,17]. The main challenge of LBEM is to select an RSS for each lumping block that can predict the fatigue damage resulting from the OSSs accurately [18]. The methodologies available to determine the wave parameters of the RSS broadly fall into three categories. The DNV and Sheehan LBEMs are the representative methods of the first category, where the wave parameters of the RSS can be determined from the wave parameters of the OSSs directly [19,20]. Referring to DNV LBEM, the summation of the interval and the largest wave height of the OSSs in the block are set to the wave height of the RSS, and the summation of the half interval and averaging wave period of the OSSs in the block are set to the wave period of the RSS [19]. In Sheehan LBEM, the largest wave height of the OSSs in the block is selected as the wave height of the RSS, and the probabilistic average of the wave period of the OSSs in the block is set to the wave period of the RSS [20]. The fatigue damage of offshore structures therefore will be overestimated significantly as the wave height of the RSS is much larger than the needed one.

In the second category, the OSSs with the same wave height/period are lumped into a block, and the wave period/height of the RSS can be calculated from the correlation of the wave period/height, the sea state's occurrence probability and a fatigue parameter, such as in the Mittendorf and Burton LBEMs [21,22]. In the Mittendorf LBEM, the OSSs with the same wave height are lumped into a block, and the probabilistic average of the wave period of the OSSs in the block is set to the wave period of the RSS [21]. In the Burton LBEM, the OSSs with the same wave period are lumped into a block, and the wave height of the RSS can be determined from the correlation of the wave height, the sea state's occurrence probability and the exponent of the S-N curve [22]. Obviously, the wave parameters of the RSS can be easily calculated for a specific lumping block case, but the accuracy of these LBEMs becomes very poor if the OSSs comprise several wave height/period intervals. Different from the Mittendorf and Burton LBEMs, the OSSs with different wave heights and periods are lumped into a block in the Jia LBEM [23]. The correlation utilized in the Burton LBEM is employed to calculate the wave height of the RSS, and the probabilistic average period principle used in the Mittendorf LBEM is adopted to determine the wave period of the RSS [23]. It has been proven that the offshore structure's fatigue damage resulting from the RSS related to the Jia LBEM method is smaller than that resulting from the OSSs.

In the third type of method, the wave parameters of the RSS are determined from the wave power spectral density (PSD) of the OSSs from the viewpoint of a fatigue damage equivalence criterion, such as in the Seidel and Song LBEMs [17,24]. In the Seidel LBEM, a compact equation between the input wave energy and fatigue damage equivalent loads on the large-diameter monopile wind turbine is derived and the relationship between the wave energy of the RSS and the OSSs can be established based on the compact equation from the viewpoint of the fatigue damage equivalence criterion. The wave height and period of the RSS can be obtained based on the equivalent wave energy and quasi-static considerations. It has been proven that the Seidel LBEM can give excellent fatigue prediction results for the structure components at the bottom of large-diameter monopile wind turbines. However, its

accuracy becomes poor if the resonance part of the response is governing or the quasi-static response is of higher importance [24]. In the Song LBEM, the relationship between the sea state's energy and the structure's fatigue damage is provided by considering the influence of the structural fatigue parameter. From the viewpoint of the fatigue damage equivalence criterion, the spectral moments of the RSS can be determined from the spectral moments of the OSSs. The wave height and period of the RSS can be obtained from the statistical relationships between the spectral moments and wave parameters in a straightforward manner [17]. The effectiveness of the Song LBEM has been proven by many researchers. However, the second spectral moment of the RSS is obtained by an analogy method rather than the fatigue damage equivalence criterion, which has an influence on the wave period of the RSS and causes this method to slightly underestimate the structure's fatigue damage in some cases [25].

The purpose of this study is to propose a wave energy equivalence-based lumping block method to efficiently and accurately predict mooring lines' fatigue damage induced by stochastic wave loads. In the proposed method, a relationship between the input wave energy and the mooring line's fatigue damage is first established by considering the effect of the mooring line's nonlinearities and the fatigue parameter, and the relationship between the wave energy distribution of the RSS and the OSSs is further derived from the fatigue damage equivalence viewpoint. Two modified statistical relationships between the spectral moments and wave parameters are provided by incorporating the effect of the spectrum's peak enhancement factor, the sea state's number in the block and the sea state's equivalence bandwidth. The spectral moments of the RSS can be determined from its wave energy distribution in a straightforward manner, and the proposed method has direct physical significance compared to the conventional LBEMs. To present the theories of the LBEMs and the results of the investigation, the manuscript is structured as follows. The details of the conventional and proposed LBEMs are introduced in Sections 2 and 3. In Section 4, the environmental conditions and the numerical model are provided. The effectiveness of the proposed method is validated with a series of case studies as presented in Section 5. Finally, the conclusions are summarized in Section 6.

2. Conventional LBEMs

To clearly illustrate the theories of the LBEMs, it is assumed that the wave scatter diagram consists of *N* OSSs, they are lumped into N_b blocks and there are n_j OSSs in the *j*th block. The significant wave height and mean up-crossing period, as well as the occurrence probability of the RSS related to the *j*th block, are characterized by H_{srj} , T_{zrj} and p_{rj} , respectively.

For different LBEMs, the equivalent occurrence probability of the RSS can be calculated with the same principle, which is identical to the sum of the occurrence probability of the OSSs in the block, as shown in Equation (1).

$$p_{rj} = \sum_{k=1}^{n_j} p_{kj} \tag{1}$$

where p_{kj} is the occurrence probability of the *k*th OSS in the *j*th block.

2.1. DNV LBEM

In DNV LBEM, the H_{srj} and T_{zrj} of the RSS can be calculated based on the following expressions [20]:

$$H_{srj} = \max_{1 \le k \le n_j} \left\{ H_{skj} \right\} + \Delta H_s \tag{2}$$

$$T_{zrj} = \frac{\sum_{k=1}^{n_{T_{zj}}} T_{zkj}}{n_{T_{zj}}} + \frac{\Delta T_z}{2}$$
(3)

where $max\{\cdot\}$ represents the maximum value in the set; ΔH_s and ΔT_z are the interval of the wave height and period in the block; $n_{T_{zj}}$ is the parameter related to the number of T_z encompassed by the *j*th block, and it can be given as

$$n_{T_{zj}} = \max_{1 \le i \le n_{H_{sij}}} \left[n_{T_{zij}} \right] \tag{4}$$

where $n_{T_{zij}}$ is the number of T_z with the same H_{sij} in the *j*th block; $n_{H_{sij}}$ is the number of H_{sij} encompassed by the *j*th block.

2.2. Sheehan LBEM

Referring to the Sheehan LBEM, the H_{srj} of the RSS is identical to the largest H_{skj} of the OSSs in the block. The T_{zrj} of the RSS is identical to the probabilistic average of the OSS's T_{zkj} in the block, and it yields [21]

$$H_{srj} = \max_{1 \le k \le n_j} \left\{ H_{skj} \right\}$$
(5)

$$\Gamma_{zrj} = \frac{\sum_{k=1}^{n_j} T_{zkj} p_{kj}}{p_{rj}}$$
(6)

2.3. Jia LBEM

According to the theory of the Jia LBEM, the H_{srj} of the RSS can be calculated based on the relation of the fatigue parameter m, H_s and p_k of the OSSs, while the T_{zrj} of the RSS can be calculated by the probabilistic average period of the OSSs and yields [16]

$$H_{srj} = \left(\frac{\sum_{k=1}^{n_j} H_{skj}^m p_{kj}}{p_{rj}}\right)^{\frac{1}{m}}$$
(7)

$$\Gamma_{zrj} = \frac{\sum_{k=1}^{n_j} T_{zkj} p_{kj}}{p_{rj}}$$
(8)

where *m* is the exponent of the fatigue curve.

2.4. Spectral Moment Equivalence (SME)-Based LBEM

In the SME LBEM, the relation of the input spectral moment and structural fatigue damage is provided, and it can be given as [18]

$$Da_k = \kappa m_{0k}^{\lambda m} \tag{9}$$

where Da_k is the cumulative fatigue damage contributed by the *k*th OSS; m_{0k} is the zeroth spectral moment of the *k*th OSS; *m* is the exponent of the fatigue curve; λ is the exponent of the zeroth spectral moment obtained via the regression algorithm, and $\lambda = 2/m$; κ is the coefficient related to the zeroth spectral moment of the input sea state and the fatigue damage of the offshore structure.

Based on the relation, the cumulative fatigue damage caused by the RSS and OSSs related to the *j*th block can be given as

$$Da_{rj} = \kappa m_{0rj}^{\lambda m} p_{rj} \tag{10}$$

$$Da_j = \kappa \sum_{k=1}^{n_j} Da_{kj} p_{kj} = \kappa \sum_{k=1}^{n_j} m_{0kj}^{\lambda m} p_{kj}$$

$$\tag{11}$$

where Da_{rj} and Da_j are the cumulative fatigue damage resulting from the RSS and OSSs related to the *j*th block; m_{0rj} is the zeroth spectral moment of the RSS related to the *j*th block; m_{0kj} is the zeroth spectral moment of the *k*th OSS in the *j*th block.

The relation of the zeroth spectral moments of the RSS and OSSs can be set up according to the fatigue equivalence criterion and yields

$$m_{0rj} = \left(\frac{\sum_{k=1}^{n_j} m_{0kj}^{\lambda m} p_{kj}}{p_{rj}}\right)^{\frac{1}{\lambda m}}$$
(12)

The second spectral moment of the RSS can be determined from the second spectral moments of the OSSs by analogy with Equation (12), and it can be given as

$$m_{2rj} = \left(\frac{\sum_{k=1}^{n_j} m_{2kj}^{\lambda m} p_{kj}}{p_{rj}}\right)^{\frac{1}{\lambda m}}$$
(13)

where m_{2rj} represents the second spectral moment of the RSS related to the *j*th block, and m_{2kj} represents the second spectral moment of the *k*th OSS in the *j*th block.

The H_{srj} and T_{zrj} of the RSS can be determined from its spectral moments directly based on the following formula:

$$H_{srj} = 4.0\delta_j \sqrt{m_{0rj}} \tag{14}$$

$$T_{zrj} = 2\pi \sqrt{\frac{m_{0rj}}{m_{2rj}}} \tag{15}$$

where δ_j is a correction factor adopted to consider the effect of the OSS's number in the *j*th block, and it has the following expression:

$$\delta_j = \left(n_j\right)^{\frac{1}{4mn_j}} \tag{16}$$

where n_i is the number of OSSs in the *j*th block, and *m* is the exponent of the fatigue curve.

3. The Novel Wave Energy Equivalence (WEE)-Based LBEM

The SME LBEM simply depends on the zeroth and second spectral moments and the occurrence probability of the OSSs, and it is therefore very easy to achieve for practicing engineers. However, the formula adopted to calculate the second spectral moment of the RSS is derived by the analogy method rather than the fatigue equivalence criterion, which makes the SME LBEM slightly non-conservative for some lumping block cases.

In this study, a novel wave energy equivalence (WEE)-based LBEM is proposed to improve the applicability of the SME LBEM. In the proposed WEE LBEM, a compact relationship between the input wave energy and the mooring lines' fatigue damage is established with the aid of a regression algorithm, and the relationship between the wave energy distribution of the RSS and OSSs is further derived from the viewpoint of the fatigue equivalence criterion. The spectral moments, including the zeroth, first and second spectral moments, of the RSS are then calculated from its wave energy distribution directly, and the wave parameters of the RSS are finally determined from the modified statistical relationships between the spectral moments and wave parameters. The flow-chart of the proposed WEE LBEM is illustrated in Figure 1.



Figure 1. Flow-chart of the proposed WEE LBEM.

3.1. The Relationship between Input Wave Energy and Mooring Lines' Fatigue Damage

Based on the Longuet-Higgins wave model, the stochastic wave can be converted into a series of regular waves with different amplitudes, frequencies and phases. This means that the wave power spectral density (PSD) of the input sea state can be discretized into a series of energy components, and each energy component can be used to construct the regular wave with different frequencies. Therefore, the relationship between the input wave energy component and the mooring lines' fatigue damage can be established for a specific wave frequency. The flow-chart of the wave PSD discretization and regular wave construction is illustrated in Figure 2. As the dynamic responses of the mooring system are very sensitive to both the H_s and T_z of the input sea state, the sea states with non-zero occurrence probability in the wave scatter diagram are selected. Furthermore, to ensure that the discrete wave frequency intervals of the wave PSD are the same for different sea states, the lower and upper limits of the wave frequency and the number of discrete wave energy components should be identical to each other. The lower and upper limits of the wave frequency can be determined based on the wave parameters of the selected sea states, and they yield

$$\omega_L = \left(-\frac{3.11}{\left[\min(H_{sk}) \right]^2 \ln(0.002)} \right)^{0.25} \tag{17}$$

$$\omega_H = \left(-\frac{3.11}{[max(H_{sk})]^2 ln(0.998)} \right)^{0.25}$$
(18)



where min(*) and max(*) represent the minimum and maximum values in the set, respectively.

Figure 2. Flow-chart of the wave PSD discretization and regular wave construction.

In this paper, the JONSWAP spectrum is adopted to depict the wave energy distribution of the selected sea state, and the wave PSD of the *k*th sea state can be given as

$$S_{\eta k}(\omega) = \frac{1.2905 H_{sk}^2 g^2}{T_{zk}^4 \omega^5} exp \left[-\frac{5}{4} \left(\frac{\omega_{pk}}{\omega} \right)^4 \right] \gamma^{exp \left[-\frac{(\omega - \omega_{pk})^2}{2\sigma^2 \omega_{pk}^2} \right]}$$
(19)

$$\begin{cases} \omega \le \omega_{pk} & \sigma = 0.07\\ \omega > \omega_{pk} & \sigma = 0.09 \end{cases}$$
(20)

$$\omega_{pk} = \frac{2\pi}{1.407(1 - 0.287ln\gamma)^{1/4}T_{zk}}$$
(21)

The wave PSD of the *k*th sea state is further discretized into *N* wave energy components according to the equal wave frequency interval principle, and the wave energy for the *i*th wave frequency ω_i can be given as

$$E_{k\omega_i} = S_{\eta k}(\omega_i) \Delta \omega \tag{22}$$

$$\Delta\omega = \frac{\omega_H - \omega_L}{N} \tag{23}$$

where *N* is the number of discretized wave energy components, and it is 50 in this paper.

The regular wave elevation associated with the *i*th wave energy component of the *k*th sea state can be given as

$$\eta_{ki} = a_{ki}\cos(k_i x - \omega_i t) \tag{24}$$

where a_{ki} is the amplitude of the regular wave and $a_{ki} = \sqrt{2E_{k\omega_i}}$; k_i is the wave number, which can be calculated based on the dispersion relation.

Repeating steps 3–5, all of the regular waves related to the selected sea states can be constructed. It should be mentioned that the mooring lines' fatigue damage under all of the constructed regular waves are estimated, but, due to space limitations, this paper presents results for only ten wave frequencies, covering almost all of the wave frequencies that may be encountered by mooring lines, as illustrated in Figures 3 and 4.



Figure 3. Illustration of wave PSD discretization.



Figure 4. Cont.



Figure 4. The relationships between the input wave energy and mooring lines' fatigue damage.

Mooring lines' tension resulting from a series of regular waves with different amplitudes and frequencies is first estimated with the fully coupled analysis model, and the mooring lines' fatigue damage is then estimated with the time-domain fatigue assessment method to fully consider the effect of the mooring line's nonlinearities. The relationships between the input wave energy and the mooring lines' fatigue damage are set up for different input sea states with the aid of a regression algorithm, and the corresponding results are presented in Figure 4a–j. In the figure, the circles represent the mooring lines' fatigue damage contributed by the regular waves related to different input sea states, while the line denotes the fitted results with the regression algorithm.

From the figure, one can find that the mooring lines' fatigue damage is proportional to the λm th of the input wave energy for different wave frequencies, and the parameter *m* is the exponent of the fatigue curve. It is notable that the exponent λ decreases slightly as the wave frequency increases, and the maximum value of the exponent λ is not larger than 3/5, as illustrated in Figure 4k. Therefore, the relationship between the input wave energy and mooring lines' fatigue damage can be characterized by a formula with a constant exponent, and the exponent λ can be set to its maximum value (i.e., 3/5) for conservative considerations.

The compact formula adopted to depict the relationship between the input wave energy and the mooring lines' fatigue damage for a specific wave frequency ω_i can be given as

$$D_{ai} = \kappa_i E_{\omega_i}^{3m/5} \tag{25}$$

where D_{ai} is the mooring lines' fatigue damage resulting from the *i*th wave energy component E_{ω_i} related to wave frequency ω_i ; κ_i is the coefficient related to wave frequency ω_i ; *m* is the exponent of the T-N curve, which is 3 for mooring lines.

3.2. The Relationship between the Wave Energy of RSS and OSSs

The fatigue damage caused by the *i*th wave energy component related to frequency ω_i in the RSS should be identical to the sum of the fatigue damage caused by the *i*th energy component with the same frequency in all of the OSSs in the block. Therefore, the *i*th wave energy component of the RSS can be obtained from the *i*th wave energy component of the OSSs in the block.

As illustrated in Figure 5, for a three-OSS block case, the relationship between the *i*th wave energy component of the RSS and the OSSs for wave frequency ω_i can be given as

$$E_{rj\omega_i} = \left(\frac{\sum_{k=1}^{n_j} E_{okj\omega_i}^{3m/5} p_{kj}}{p_{rj}}\right)^{\frac{3}{3m}}$$
(26)

where $E_{rj\omega_i}$ is the *i*th wave energy component related to wave frequency ω_i of the *j*th RSS; $E_{okj\omega_i}$ is the *i*th wave energy component related to wave frequency ω_i of the *k*th OSS in the *j*th block, and $E_{okj\omega_i} = S_{\eta kj}(\omega_i)\Delta\omega$, where $S_{\eta kj}$ is the wave PSD of the *k*th OSS in the *j*th block and $\Delta\omega$ is the wave frequency discretization interval of the wave PSD; p_{kj} is the occurrence probability of the *k*th OSS in the *j*th block; n_j is the number of OSSs in the *j*th block; p_{rj} is the occurrence probability of the *j*th RSS (see Equation (1)).



Figure 5. Illustration of the relationship between wave energy of the RSS and OSSs related to wave frequency ω_i . (a) Wave PSD for original sea state 1, (b) Wave PSD for original sea state 2, (c) Wave PSD for original sea state 3, (d) Wave PSD for representative sea state.

The wave energy distribution of the RSS can be obtained by applying Equation (18) to all of the wave frequencies of the OSS in the block. Then, the spectral moments of the *j*th RSS, including the zeroth, first and second spectral moments, can be calculated based on its wave energy distribution directly, yielding

$$m_{0rj} = \sum_{i=1}^{n_s} E_{rj\omega_i} \tag{27}$$

$$m_{1rj} = \sum_{i=1}^{n_s} \omega_i E_{rj\omega_i} \tag{28}$$

$$m_{2rj} = \sum_{i=1}^{n_s} \omega_i^2 E_{rj\omega_i} \tag{29}$$

where m_{0rj} , m_{1rj} and m_{2rj} are the zeroth, first and second spectral moments of the RSS, and n_s is the number of discretization intervals of the RSS's wave PSD.

3.3. The Modified Statistical Relationships between Spectral Moments and Wave Parameters

The wave parameters of the RSS, including the H_{srj} and T_{zrj} , can be calculated based on the statistical relationships between the spectral moments and wave parameters. However, many researchers have shown that the fatigue damage contributed by the RSS is closely related to the number of OSSs in the block and the bandwidth of the RSS [17,25]. According to the commonly utilized statistical relationship, the significant wave height is only related to the zeroth spectral moment, which can be determined from the energy distribution. Therefore, the parameters that have an influence on the energy distribution of the RSS should be introduced to construct the correction factor for the significant wave height. Firstly, the wave energy distribution of the OSS is closely related to the wave spectrum and the spectrum's enhancement factor, and the spectrum's enhancement factor has a significant influence on the energy distribution of the RSS in turn. Therefore, the effect of the spectrum's peak enhancement factor should be taken into consideration.

Secondly, the lumping block usually contains several OSSs, and the energy distribution is different for OSSs with different significant wave heights and up-crossing periods. The energy distribution of the RSS determined from the energy distribution of the OSS is usually a broadband spectrum, which cannot be depicted by the standard wave spectrum. The difference between the calculated wave energy distribution and the theoretical wave energy distribution depicted by a standard wave spectrum can be characterized by the equivalent bandwidth. Therefore, the effect of the equivalent bandwidth of the RSS should be considered.

Thirdly, the significant wave heights and up-crossing periods encompassed by the block increase as the number of OSSs in the block increases, and the fatigue damage contributed by the low-frequency cycles associated with the OSS with very long periods, and the high-frequency cycles associated with the OSSs with very small periods, will be suppressed by the representative sea state. Therefore, a factor related to the number of original sea states in the block should be introduced to amplify the wave energy distribution to obtain conservative results.

Since it is a modification of the commonly utilized statistical relationship, the correction factor should vary slightly with the input parameters. In this paper, the correction factor α_j related to the spectrum's peak enhancement factor γ , the number of OSSs n_j and the bandwidth of the RSS λ_{2j} are introduced for the H_{srj} of the RSS in exponent form, and it can be given as

$$\alpha_j = (1 - 0.0145\gamma)\lambda_{2j} \left(\frac{n_j}{N}\right)^{0.0145/\gamma} \tag{30}$$

where *N* is the number of OSSs in the wave scatter diagram; λ_{2j} is the equivalent bandwidth of the *j*th RSS and $\lambda_{2j} = \frac{\sqrt{m_{0rj}m_{2rj}}}{m_{1rj}}$; the coefficient "0.0145" is obtained from the simulation data via the regression algorithm.

As illustrated in Figure 6, the correction factor α_j increases slightly as the number of OSSs in the *j*th block increases. In addition, the H_{srj} of the RSS calculated based on the modified statistical relationship is slightly larger than those calculated based on the commonly utilized statistical relationship (i.e., $H_{srj} = 4.0\sqrt{m_{0rj}}$), and the discrepancies between them increase as the number of OSSs increases. It should be mentioned that the correction factor α_j will approach 1.0 as the number of OSSs decreases to 1, and the H_{srj} of the RSS will approach the H_{skj} of the OSS.

The T_{zrj} of the RSS calculated based on the commonly utilized statistical relationship (i.e., $T_{zrj} = 2\pi \sqrt{m_{0rj}/m_{2rj}}$) is close to the target ones when the spectrum's enhancement factor γ is identical to 1.0, as presented in Figure 7a. However, the T_{zrj} of the RSS determined from the commonly utilized statistical relationship is smaller than the target ones when the spectrum's enhancement factor γ is larger than 1.0, and the discrepancies between them increase as the parameter γ increases, as illustrated in Figure 7b. As the up-crossing period is very close to the cycle number of the response, which is very important for fatigue damage, the spectrum's peak enhancement factor should be incorporated into the correction factor to allow the calculated up-crossing period to match the target ones.



Figure 6. Relation of the correction factor α_i and the number of OSSs in the block.



Figure 7. The relation of correction factor β_j and spectrum's peak enhancement factor. (a) Wave period calculated by the commonly utilized statistical relationship with γ of 1.0, (b) Wave period calculated by the commonly utilized and modified statistical relationships with different γ value.

In this study, a correction factor β_j related to the spectrum's peak enhancement factor γ is introduced for the T_{zrj} of the RSS, and it can be given as

$$\beta_i = 1 + 0.0072(\gamma - 1) \tag{31}$$

where coefficient "0.0072" is obtained from the simulation data via the regression algorithm.

As illustrated in Figure 7b, after considering the effect of the spectrum's peak enhancement factor γ , the T_{zrj} of the RSS determined from the modified statistical relationship are in perfect agreement with the target ones, and the correction factor will approach 1.0 as the spectrum's enhancement factor decreases to 1.0.

3.4. Wave Parameters of the RSS

The H_{sri} and T_{zri} of the RSS can be determined from the following formulae:

$$H_{srj} = 4.0\alpha_j \sqrt{m_{0rj}} \tag{32}$$

$$T_{zrj} = 2\pi\beta_j \sqrt{m_{0rj}/m_{2rj}} \tag{33}$$

The WEE LBEM simply depends on the wave energy and occurrence probability of the OSSs in the block, which is very easy achieve for practicing engineers. In addition, the spectral moments adopted to calculate the wave parameters of the RSS can be obtained from its wave energy distribution directly, which gives the proposed LBEM more physical significance compared to the conventional LBEMs.

4. Numerical Model and Environmental Conditions

4.1. Numerical Model

The semi-submersible (SEMI) platform adopted in Song and Wang's research is adopted in this study to investigate the effectiveness of the proposed WEE LBEM [11,18]. As shown in Figure 8a, the SEMI is a deep-draft platform where four large columns are connected together with a ring pontoon. The SEMI operates at a water depth of 1828.8 m and is positioned with 12 mooring lines. These mooring lines are composed of three different components and grouped into four bundles that connect to the SEMI with the fairleads at each corner, as illustrated in Figure 8b. The main specifications of the SEMI and the mooring lines are summarized in Tables 1 and 2, respectively.



Figure 8. Illustration of the SEMI: (**a**) semi-submersible platform; (**b**) layout of mooring system; (**c**) dynamic analysis model.

Table 1. Main specifications of the SEMI.

Item	Value	Item	Value
Column side (m)	17.069	Pontoon length (m)	48.768
Column height (m)	59.131	Pontoon width (m)	12.192
Column corner radius (m)	2.438	Pontoon height (m)	9.754
Operation draft (m)	37.795	Pontoon corner radius (m)	1.219
Freeboard (m)	21.336	Radius of gyration in roll (m)	36.354
Vertical of COG (m)	24.465	Radius of gyration in pitch (m)	36.354
Total displacement (t)	63,811	Radius of gyration in yaw (m)	45.785

The numerical simulations are conducted in the DNV-SESAM software with the coupled dynamic analysis model, as illustrated in Figure 8c. In the numerical simulation, the head wave is defined as propagating along the *x*-axis, and the simulation duration is set to 10,800 s (i.e., 3 h) with a time step of 0.1 s for each study case.

Table 2. Main specifications of mooring lines.

Item	Platform Chain	Wire Rope	Anchor Chain
Length of lines (m)	183	2560	305
Diameter of lines (m)	0.147	0.273	0.147
Wet density of lines (kg/m)	360.47	57.58	360.47
Axial stiffness of lines (kN/m)	1,845,400	81,237	1,845,400
Minimum breaking load (kN)	19,089	12,705	19,089

4.2. Environmental Conditions

In engineering practice, the wind and current loads on SEMI are usually simplified as a constant load, which generally causes the mean offset of the SEMI, and they have little contribution to the structure's fatigue damage [4]. Therefore, only the wave hydrodynamic loads on the SEMI are of concern in this study. To estimate the cumulative fatigue damage of the mooring lines occurring at the fairleads of SEMI, the long-term wave condition should be considered, which is usually discretized into a series of short-term sea states represented by the wave scatter diagram in engineering practice. A typical wave scatter diagram in the North Sea, which includes 197 short-term sea states, is illustrated in Table 3.

									$T_z(s)$)								
$H_{s}(m)$	1.5	2.5	3.5	4.5	5.5	6.5	7.5	8.5	9.5	10.5	11.5	12.5	13.5	14.5	15.5	16.5	17.5	18.5
0.5	0	0	13	1337	8656	11,860	6342	1863	369	56	7	1	0	0	0	0	0	0
1.5	0	0	0	293	9860	49,760	77,380	55,697	23,757	7035	1607	305	51	8	1	0	0	0
2.5	0	0	0	22	1975	21,588	62,300	74,495	48,604	20,660	6445	1602	337	63	11	2	0	0
3.5	0	0	0	2	349	6955	32,265	56,750	50,991	28,380	11,141	3377	843	182	35	6	1	0
4.5	0	0	0	0	60	1961	13,543	32,885	38,575	26,855	12,752	4551	1309	319	69	13	2	0
5.5	0	0	0	0	10	510	4984	16,029	23,727	20,083	11,260	4636	1509	410	97	21	4	1
6.5	0	0	0	0	2	126	1670	6903	12,579	12,686	8259	3868	1408	422	109	25	5	1
7.5	0	0	0	0	0	30	521	2701	5944	7032	5249	2767	1117	367	102	25	6	1
8.5	0	0	0	0	0	7	154	979	2559	3506	2969	1746	776	277	84	22	5	1
9.5	0	0	0	0	0	2	43	332	1019	1599	1522	992	483	187	61	17	4	1
10.5	0	0	0	0	0	0	12	107	379	675	717	515	273	114	40	12	3	1
11.5	0	0	0	0	0	0	3	33	133	266	314	247	142	64	24	7	2	1
12.5	0	0	0	0	0	0	1	10	44	99	128	110	68	33	13	4	1	0
13.5	0	0	0	0	0	0	0	3	14	35	50	46	31	16	7	2	1	0
14.5	0	0	0	0	0	0	0	1	4	12	18	18	13	7	3	1	0	0
15.5	0	0	0	0	0	0	0	0	1	4	6	7	5	3	1	1	0	0
16.5	0	0	0	0	0	0	0	0	0	1	2	2	2	1	1	0	0	0

Table 3. A typical wave scatter diagram in the North Sea.

4.3. Wave Scatter Diagram Discretization

To examine the applicability of the proposed WEE LBEM to different lumping block partition cases, the OSSs in the wave scatter diagram illustrated in Table 3 are lumped into 57, 29, 15 and 6 blocks according to a specific rule, respectively. Taking the 57-block case, for example, OSSs in adjacent rows and columns are lumped into a block, as illustrated in Figure 9a. Similarly, OSSs in adjacent 3 rows and 3 columns, 4 rows and 4 columns, 6 rows and 8 columns are lumped into a block for 29, 15 and 6 block cases, as illustrated in Figure 9b–d, respectively. Compared with the full wave scatter diagram, the computational effort of the fatigue damage assessment can be reduced by 3.46, 6.79, 13.13 and 32.83 times when the OSSs in the wave scatter diagram are lumped into 57, 29, 15 and 6 blocks, respectively.

$H_s \setminus T_z$	1	2	3 4	5 6	7 8	9 10	11 12	13 14	15 16
1	13	1337	8656 11,860	6342 1863	369 56	7 1	0 0	0 0	0 0
2	0	293	9860 4 9,760	77,380 355,697	23,757 7035	1607 305	51 <mark>6</mark> 8	1 7 0	0 0
3	0	22	1975 21,588	62,300 74,495	48,604 20,660	6445 1602	337 63	11 2	0 0
4	0	2	349 6955	32,265 56,750	50,991 28,380	11,141 3377	843 182	35 6	1 15 0
5	0	0	60 1961	13,543 32,885	38,575 26,855	12,752 4551	1309 319	69 13	2 0
6	0	0	10 510	4984 16,029	23,727 20,083	11,260 4636	1509 4 10	97 21	4 1
7	0	0	2 126	1670 6903	12,579 12,686	8259 3868	1408 422	109 25	5 1
8	0	0	0 30	521 2701	5944 7032	5249 2767	1117 ²⁷ 367	102 25	6 1
9	0	0	0 7	154 979	2559 3506	2969 1746	776 277	84 22	5 1
10	0	0	0 2	43 332	1019 1599	1522 992	483 187	61 17	30 4 1
11	0	0	0 0	12 107	379 675	717 515	273 114	40 12	3 1
12	0	0	0 0	3 33	133 266	314 247	142 64	24 7	2 1
13	0	0	0 0	1 10	44 99	128 110	68 33	13 4	1 0
14	0	0	0 0	0 3	14 35	50 46	31 16	7 2	40 1 0
15	0	0	0 0	0 49 1	4 12	18 18	13 7	3 1	0 0
16	0	0	0 0	0 0	1 4	6 7	5 3	53 1 1	0 0
17 (8	l) ₀	0	0 0	0 0	0 54 1	2 55 2	2 56 1	1 57 0	0 0

Figure 9. Cont.

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
1	13.	1337	8656	11 860	6342	1863	369	56	7	10	0	0	0	0	0	0
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2	0	293	9860	49,760	77,380	55,697	23,757	7035	1607	305	51	5	1	0	0	0
3	0	22	1975	21,588	62,300	74,495	48,604	20,660	6445	1602	337	63	11	2 <mark>6</mark>	0	0
4	0	2	349	6955	32,265	56,750	50,991	28,380	11,141	3377	843	182	35	6	1	0
5	0	0 7	60	1961	13,543	32,885	38,575	26,855	9 12,752	4551	1309	10 319	69	13	2	0
6	0	0	10	510	4984	16,029	23,727	20,083	11,260	4636	1509	410	97	21	4	1
7	0	0	2	126	1670	6903	12,579	12,686	8259	3868	1408	422	109	25	5	1
	0	0	0 1	2 30	521	13 2701	5944	7032	14 ₅₂₄₉	2767	1117	15 ₃₆₇	102	25	16 6	1
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11	0	0	0	0	12	107	379	675	717	515	273	114	40	12	3	1
12	0	0	0	0	3	33	133	266	314	247	142	64	24	7	2	1
13	0	0	0	0	1	10	44	99	128	110	68	33	13	4	1	0
14	0	0	0	0	0	22 ₃	14	35	23 50	46	31	24 16	7	2	25 1	0
15	0	0	0	0	0	1	4	12	18	18	13	7	3	1	0	0
16	0	0	0	0	0	0		4	6		5	3	1		1 0	י נ
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17	· U	U	U	U	U	U	U		2	2	2	1	1	0	U	U
$H_s \setminus T_z$	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
1	13	1337	8656	11,860	6342	1863	369	56	7	1	0	0	0	0	0	0
2	0	293	9860	49,760	77,380	55,697	23,757	7035	1607	305	51	8	1	0	0	
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4	0	2	349	6955	32,265	56,750	50,991	28,380	11,141	3377	843	182	35	6	1	
5	0	0	60	1961	13,543	32,885	38,575	26,855	12,752	4551	1309	319	69	13	2	0
6	0	0	10	510	4984	16,029	23,727	20,083	11,260	4636	1509	410	97	21	4	1
7	0	0	2	126	1670	6903	12,579	12,686	8259	3868	1408	422	109	25	o 5	1
8	0	0	0	30	521	2701	5944	7032	5249	2767	1117	367	102	25	6	1
9	0	0	0	7	154	979	2559	3506	2969	1746	776	277	84	22	5	1
10	0	0	0	9,	43	332	1019	1599	1522	992	483	187	61	17	4	1
10	0	0	0		10	107	10 270	(75	717	515	11	114	40	12	12	
11	0	0	0	0	12	107	3/9	6/5	717	515	2/3	114	40	12	3	1
12	0	0	0	0	3	33	133	266	314	247	142	64	24	7	2	1
13	0	0	0	0	1	10	44	99	128	110	68	33	13	4	1	0
14	0	0	0	0	0	3	14	35	50	46	31	16	7	2	1	0
15	0	0	0	0	0	1	4	12	18	18	13	7	3	15	0	0
16	0	0	0	0	0	0	1	4	6	7	5	3	1	1	0	0
17 (0) ₀	0	0	0	0	0	0	1	2	2	2	1	1	0	0	0
L_*′	-	~		,	<u> </u>	v							<u> </u>			<u> </u>
$H_s \setminus T_z$	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
1	13	1337	8656	11,860	6342	1863	369	56	7	1	0	0	0	0	0	0
2	0	293	9860	49,760	77,380	55,697	23,757	7035	1607	305	51	8	1	0	0	0
3	0	22	1975	21,588	62,300	74,495	48,604	20,660	6445	1602	337	63	11	2	0	0
4	0	2	349	6955	32,265	56,750	50,991	28,380	11,141	3377	843	182	35	6	1	0
5	0	0	60 1	1961	13.543	32.885	38.575	26.855	12.752 2	4551	1309	319	69	13	3 2	0
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6	U A	0	10	510	4704	10,029	10	20,003	11,200	4000	1309	410	7/	21	4	1
7	0	0	2	126	1670	6903	12,579	12,686	8259	3868	1408	422	109	25	5	1
8	0	0	0	30	521	2701	5944	7032	5249	2767	1117	367	102	25	6	1
9	0	0	0	7	154	979	2559	3506	2969	1746	776	277	84	22	5	1
10	0	0	0	2	43	332	1019	1599	1522	992	483	187	61	17	4	1
11	0	0	0	0	12	107	379	675	717	515	273	114	40	12	3	1
12	0	0	0	0	3	33	133	266	314	247	142	64	24	7	2	1
12	0	0	0		1	10	44	00	120	110	60	22	12	1	6	
13	U	U	U	0	1	10	44	77	128	110	60	33	13	4	1	U
14	0	0	0	0	0	3	14	35	50	46	31	16	7	2	1	0
15	0	0	0	0	0	1	4	12	18	18	13	7	3	1	0	0
16	0	0	0	0	0	0	1	4	6	7	5	3	1	1	0	0
1 (d	D _							_				П.	1	0		

Figure 9. Wave scatter diagram discretization: (a) 57-block case; (b) 29-block case; (c) 15-block case; (d) 6-block case.

5. Results and Discussion

To examine the effectiveness of the proposed WEE LBEM, two different scenarios are analyzed. The first scenario is that the JONSWAP spectrum is adopted to depict the wave energy distribution of the input sea state, where the spectrum's peak enhancement factor is set to 3.3, and the performance of the conventional and proposed LBEMs are evaluated. The second scenario is that the Pierson–Moskowitz (P-M) spectrum is adopted to depict the wave energy distribution of the input sea state, where the spectrum's peak enhancement factor is set to 1.0, and the applicability of the proposed WEE LBEM to various spectra is investigated.

5.1. Wave Parameters of the RSS

The wave parameters of the RSS are first calculated based on these five LBEMs, and the six-block case is selected to demonstrate the discrepancies in the wave parameters determined from different LBEMs. The comparison results related to the JONSWAP spectrum with γ of 3.3 are listed in Tables 4 and 5, respectively.

Blocks	DNV LBEM/m	Sheehan LBEM/m	Jia LBEM/m	SME LBEM/m	WEE LBEM/m
Block 1	8.500	7.500	3.237	3.561	3.584
Block 2	8.500	7.500	4.717	4.993	5.238
Block 3	8.500	7.500	6.014	6.206	6.275
Block 4	15.500	14.500	9.044	9.240	9.291
Block 5	17.500	16.500	9.448	9.610	10.276
Block 6	17.500	16.500	10.061	10.289	10.506

Table 4. H_{srj} of the RSS resulting from different LBEMs.

Table 5. *T*_{*zri*} of the RSS resulting from different LBEMs.

Blocks	DNV LBEM/s	Sheehan LBEM/s	Jia LBEM/s	SME LBEM/s	WEE LBEM/s
Block 1	6.500	7.692	7.692	7.899	8.083
Block 2	12.000	10.291	10.291	10.217	10.681
Block 3	17.000	14.792	14.792	14.544	14.905
Block 4	8.000	8.363	8.363	8.200	8.368
Block 5	12.000	11.164	11.164	10.786	11.271
Block 6	17.000	14.921	14.921	14.648	15.044

As can be seen from Table 4, the discrepancies in the significant wave heights resulting from the different LBEMs are remarkable for a specific block. Taking block 1, for example, the significant wave heights determined from the DNV, Sheehan, Jia, SME and WEE methods are 8.500 m, 7.500 m, 3.237 m, 3.561 m and 3.584 m, respectively. The significant wave height determined from the DNV LBEM is slightly larger than that resulting from the Sheehan LBEM, but both of them are much larger than those calculated by the other three LBEMs. While the significant wave height determined from the Jia LBEM is slightly smaller than that calculated by the SME LBEM, both of them are slightly smaller than that resulting from the WEE LBEM. Similar variation trends can be found for the other five blocks.

From Table 5, one can find that the up-crossing periods determined from the DNV LBEM are smaller than those determined from the Sheehan LBEM for the block 1 and 4 cases, but larger than those resulting from the Sheehan LBEM for the block 2, 3, 5 and 6 cases. The up-crossing periods calculated by the Sheehan LBEM are identical to those calculated by the Jia LBEM for the same theories used to calculate the up-crossing periods. The up-crossing period calculated by the Sheehan and Jia LBEMs is slightly smaller than that calculated by the SME LBEM for the block 1 case, but larger than those calculated by the SME LBEM for the block 2–6 cases. It is notable that the up-crossing periods resulting from the WEE LBEM are slightly larger than those resulting from the Sheehan, Jia and SME LBEMs.

To examine the applicability of the proposed WEE LBEM to various wave spectra, the wave parameters of the RSS related to the JONSWAP and P-M spectra are calculated, and the results are presented in Figure 10. From the figure, one can find that the spectrum's peak enhancement factor γ has a slight influence on the wave parameters of the RSS. The H_{srj} of the RSS calculated by the proposed WEE LBEM with $\gamma = 1.0$ is slightly larger than that calculated by the proposed WEE LBEM with $\gamma = 1.0$ is slightly smaller than that calculated by the proposed WEE LBEM with $\gamma = 1.0$ is slightly smaller than that calculated by the proposed WEE LBEM with $\gamma = 3.3$. Similar variation trends are found for the RSSs for the 57-, 29- and 15-block cases, but the results are not shown here due to space limitations.



Figure 10. Wave parameters of the RSSs calculated by the proposed WEE LBEM related to different wave spectra.

5.2. Fatigue Damage of Mooring Lines Related to the JONSWAP Spectrum

The fatigue damage of mooring lines accumulated at the fairleads of SEMI under the RSSs and OSSs is estimated via the time-domain fatigue assessment method. To clearly illustrate the performance of the LBEMs, the mooring lines' fatigue damage contributed by the RSSs is normalized to the fatigue damage contributed by all the OSSs in the wave scatter diagram. The normalized fatigue damage of mooring lines 4 and 6 related to the JONSWAP spectrum is summarized in Tables 6 and 7, respectively.

 Table 6. Normalized fatigue damage of mooring line 4 resulting from different LBEMs related to JONSWAP spectrum.

Plash Partition	Method								
Cases	DNV LBEM	Sheehan LBEM	Jia LBEM	SME LBEM	WEE LBEM				
57 Blocks	2.4776	1.2933	0.9639	1.0407	1.0480				
29 Blocks	3.5750	1.6673	0.9235	0.9802	1.0807				
15 Blocks	4.6778	2.2025	0.9147	0.9909	1.1366				
6 Blocks	14.1194	6.1259	0.8611	0.9812	1.1228				

Due to the normalization, the analysis indicates that the LBEM underestimates the mooring line's fatigue damage if the normalized fatigue damage is smaller than one. Otherwise, this means that the LBEM overestimates the mooring line's fatigue damage. It is worth noting that the fatigue damage estimated by the T-N curve may be smaller than the actual fatigue damage. If the fatigue damage is further underestimated by the lumping block equivalent method, the offshore structure will be in a dangerous condition. The underestimation of fatigue damage is undesirable in engineering practice.

	Method								
Cases	DNV LBEM	Sheehan LBEM	Jia LBEM	SME LBEM	WEE LBEM				
57 Blocks	2.4801	1.3005	0.9652	1.0460	1.0487				
29 Blocks	3.5769	1.6851	0.9277	0.9888	1.0856				
15 Blocks 6 Blocks	4.7424 14.8135	2.2180 6.3071	$0.9104 \\ 0.8624$	0.9920 0.9861	$1.1318 \\ 1.1241$				

Table 7. Normalized fatigue damage of mooring line 6 resulting from different LBEMs related to JONSWAP spectrum.

It can be seen from the tables that the fatigue damage of the mooring lines resulting from these five LBEMs shows similar characteristics for various mooring lines. The mooring lines' fatigue damage resulting from the DNV LBEM is much larger than the benchmark value. Taking mooring line 4, for example, the overestimation level of the DNV LBEM is approximately 147.76% for the 57-block case, and it reaches approximately 1311.94% for the six-block case. As a result, the DNV LBEM overestimates the mooring lines' fatigue damage significantly. In addition, the mooring lines' fatigue damage resulting from the Sheehan LBEM is much larger than the benchmark value, and the overestimation level increases from 29.33% to 512.59% as the number of partitioned blocks in the wave scatter diagram decreases from 57 to 6. Although the Sheehan LBEM overestimates the mooring lines' fatigue damage remarkably as well, its overestimation level is much smaller than that of the DNV LBEM. The reason for this phenomenon is that the effect of the sea state's occurrence probability is taken into consideration in the process of determining the wave parameters of the RSS and a smaller H_{srj} is selected.

Different from the DNV and Sheehan LBEMs, the mooring lines' fatigue damage resulting from the Jia LBEM is consistently smaller than the benchmark value. This means that the Jia LBEM always underestimates the mooring lines' fatigue damage. When the number of OSSs in the block increases, the underestimation level of the Jia LBEM increases significantly, and the largest underestimation level can reach 13.89%. In contrast to the Jia LBEM, the mooring line fatigue damage related to the SME LBEM is slightly larger than the benchmark value for the 57-block case, but it is slightly smaller than the benchmark value for the 29-, 15- and 6-block cases. Taking mooring line 4, for example, the overestimation and underestimation levels of the SME LBEM are 4.07%, 1.98%, 0.91% and 1.88% for the 57-, 29-, 15- and 6-block cases, respectively. The underestimation level of the SME LBEM is much smaller than that of the Jia LBEM. This means that the SME LBEM gives more accurate fatigue damage predictions than the Jia LBEM.

A noteworthy observation is that the mooring lines' fatigue damage contributed by the proposed WEE LBEM is close to and always larger than the benchmark values. Different from the DNV and Sheehan LBEMs, the overestimation level of the proposed WEE LBEM increases slightly as the number of OSSs in the block increases, and the largest overestimation level is approximately 10%. In contrast to the Jia and SME LBEMs, the mooring lines' fatigue damage related to the proposed WEE LBEM is consistently larger than the benchmark value, and it can maintain its accuracy for different lumping block cases. These characteristics show that the proposed WEE LBEM yields the most accurate fatigue damage prediction, and it has robustness to different lumping block partitions.

5.3. Fatigue Damage of Mooring Lines Related to the P-M Spectrum

To fully investigate the applicability of the proposed WEE LBEM, the mooring lines' fatigue damage related to the P-M spectrum is further estimated with the time-domain fatigue assessment method. Similar to the results related to the JONSWAP spectrum, the fatigue damage of mooring lines resulting from the RSSs is normalized to the fatigue damage contributed by all of the OSSs in the wave scatter diagram, and the normalized fatigue damage for mooring lines 4 and 6 is listed in Tables 8 and 9, respectively.

	Method								
Cases	DNV LBEM	Sheehan LBEM	Jia LBEM	SME LBEM	WEE LBEM				
57 Blocks	2.4485	1.2663	0.9639	1.0501	1.0193				
29 Blocks	3.4050	1.5877	0.9169	0.9761	1.0361				
15 Blocks	4.7910	2.0190	0.8777	0.9280	1.0539				
6 Blocks	13.6143	5.3221	0.7469	0.8262	1.0043				

Table 8. Normalized fatigue damage of mooring line 4 resulting from different LBEMs related to P-M spectrum.

Table 9. Normalized fatigue damage of mooring line 6 resulting from different LBEMs related to P-M spectrum.

Plash Partition	Method								
Cases	DNV LBEM	Sheehan LBEM	Jia LBEM	SME LBEM	WEE LBEM				
57 Blocks	2.4491	1.2754	0.9645	1.0546	1.0222				
29 Blocks	3.4184	1.6093	0.9187	0.9807	1.0394				
15 Blocks	4.7586	2.0653	0.8833	0.9387	1.0640				
6 Blocks	14.2487	5.5122	0.7554	0.8371	1.0137				

There are five features worthy of attention in Tables 8 and 9. First, the mooring lines' fatigue damage contributed by the five LBEMs shows similar variation trends for different mooring lines. Second, the DNV and Sheehan LBEMs still overestimate the mooring lines' fatigue damage remarkably. When the number of OSSs in the block increases, the overestimation level increases dramatically, showing similar characteristics to the results under the JONSWAP spectrum.

Third, the Jia LBEM still underestimates the mooring lines' fatigue damage. When the number of OSSs in the block increases, the underestimation level increases significantly, and the largest underestimation level is approximately 25.31%, which is much larger than that related to the JONSWAP spectrum. Fourth, the SME LBEM overestimates the mooring lines' fatigue damage for the 57-block case, but it underestimates the mooring line fatigue damage for the 29-, 15- and 6-block cases. When the number of OSSs in the block increases, the underestimation level increases, and the largest underestimation level is approximately 17.38%, which is much smaller than the results related to the JONSWAP spectrum.

The most important feature is that the proposed WEE LBEM yields the most accurate and smallest conservative fatigue damage prediction for the mooring lines among these five LBEMs. When the number of OSSs in the block increases, the overestimation level of the proposed WEE LBEM increases slightly. Taking mooring line 6 as an example, the overestimation levels of the proposed WEE LBEM are 2.22%, 3.94%, 6.40% and 1.37% for the 57-, 29-, 15- and 6-block cases, respectively.

To further examine the applicability of the proposed WEE LBEM to various wave spectra, the normalized fatigue damage of mooring lines related to the JONSWAP and P-M spectra are compared, and the results are presented in Figure 11.

It can be seen from the figure that the fatigue damage of the leeward mooring lines (e.g., mooring lines 1 and 3) related to the P-M spectrum is slightly larger than that related to the JONSWAP spectrum for the 57- and 6-block cases, while the fatigue damage of the leeward mooring lines related to the P-M spectrum is slightly smaller than that related to the JONSWAP spectrum for the 29- and 15-block cases. However, the discrepancies in the mooring lines' fatigue damage related to the JONSWAP and P-M spectra are negligible.

One can also find that the fatigue damage of the windward mooring lines (e.g., mooring lines 4 and 6) related to the P-M spectrum is slightly smaller than the results related to the JONSWAP spectrum for all the lumping block cases. When the number of OSSs in the block increases, the discrepancies in the fatigue damage related to the P-M and JONSWAP spectra increase slightly. This indicates that the correction factor related to the spectrum's peak enhancement factor adopted in the proposed WEE LBEM makes it applicable to different spectra, and these qualities make the proposed WEE LBEM a useful tool for the fatigue damage assessment of the mooring system in its preliminary design stage.



Figure 11. Normalized fatigue damage of mooring lines contributed by the proposed WEE LBEM related to the JONSWAP and P-M spectra.

6. Conclusions

This paper presents a novel LBEM from the viewpoint of wave energy equivalence to efficiently and accurately estimate a mooring line's fatigue damage at the preliminary design stage. In the proposed method, a compact relationship between the input wave energy and the mooring line's fatigue damage and a modified statistical relationship between the wave parameters and spectral moments are provided via the regression algorithm. The wave energy distribution of the RSS can be obtained from the wave energy distribution of the OSS based on the compact relationship, the spectral moment of the RSS can be calculated from its wave energy distribution directly, and the wave parameters of the RSS can be determined from the modified statistical relationships easily. The effectiveness of the proposed WEE LBEM has been numerically investigated with the moored SEMI. According to this study, several conclusions can be drawn as follows.

(1) The DNV and Sheehan LBEMs consistently overestimate mooring lines' fatigue damage significantly, and the overestimation level increases dramatically as the number of OSSs in the block increases. The overestimation level of the DNV LBEM is much larger than that of the Sheehan LBEM, and the largest overestimation level of the DNV LBEM can reach 1311.94%.

(2) The Jia LBEM consistently underestimates mooring lines' fatigue damage, while the SME LBEM underestimates mooring lines' fatigue damage for some cases. The underestimation level of these two methods increases as the number of OSSs in the block increases, but the underestimation level of the SME LBEM is much smaller than that of the Jia LEMB for the same lumping block case. The largest underestimation level of the Jia LBEM can reach 13.89%. (3) The proposed WEE LBEM can yield the most accurate but slightly conservative fatigue damage predictions, and the largest overestimation level is approximately 10% for all the cited scenarios. It has excellent performance for different input wave spectra and is applicable to different lumping block cases, and it outperforms the conventional LBEM both in accuracy and robustness.

The proposed WEE LBEM will be a powerful tool for mooring line fatigue damage assessment in the preliminary stage of design, where a parameter study may be required and the costs of a time-domain fatigue assessment for a full wave scatter diagram are prohibitive.

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