

Article

A Spatial Structure Variable Approach to Characterize Storm Events for Coastal Flood Hazard Assessment

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Abstract: Over the last decades, the evaluation of hazards and risks associated with coastal flooding has become increasingly more important in order to protect population and assets. The general purpose of this research was to assess reliable coastal flooding hazard maps due to overflow and wave overtopping. This paper addresses the problem of defining credible joint statistics of significant wave heights H_s and water levels ζ , focusing on the selection of the sample pair that characterizes each sea storm, to evaluate the occurrence probability of extreme events. The pair is selected maximizing a spatial structure variable, i.e., a linear combination of H_s and ζ , specific to each point of the area at risk. The structure variable is defined by the sensitivity of the flooding process to H_s and ζ , as found by analyzing a set of inundation maps produced through a Simplified Shallow-Water numerical model (SSW). The proposed methodology is applied to a coastal stretch in the Venetian littoral (Italy), by means of a 30 year-long time series recorded at the “Acqua Alta” oceanographic research tower, located in the Northern Adriatic Sea in front of the Venetian lagoon. The critical combination of H_s and ζ forming the structure variable is presented in a map, and it can be related to the topography and the presence of mitigation measures. The return period associated with the two recent large storms that occurred in this area in 2018 and 2019 is also investigated. The proposed procedure gives credible occurrence probabilities for these events, whereas other approaches would consider them extremely unlikely.

Keywords: coastal flooding map; bivariate analysis; sampling procedure; wave height; sea level



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1. Introduction

Coastal flooding is frequently associated with two mechanisms occurring during a sea storm: overflow and wave overtopping. In the first case, the freeboard of the local levee/defense structure is steadily below the water level, whereas in the second case the flood volumes depend on the oscillating nature of the waves. They are often threatened as separate mechanisms although waves and water levels clearly affect each other. The local water level in front of the coast is a combination of offshore sea level and wave setup, influenced by the breaking process. Similarly, the wave characteristics are affected by the water level during propagation, and hence these phenomena are interrelated.

The main motivation of this paper is related to the challenges posed by the appropriate definition of sample pairs to be used for bivariate (significant wave heights H_s – water levels ζ) statistical analysis, in the framework of coastal flooding hazard assessment.

Bivariate statistical methods are significantly affected by the approach used for the selection of just two lumped values (H_s , ζ) characterizing each stormy event.

Several authors (Coles and Tawn [1], Zachary et al. [2], Masina et al. [3], Li et al. [4], Hawkes et al. [5]) analyzed engineering problems considering a sample formed by the load maxima within the storm and assuming that these maxima are simultaneous. This procedure overlooks the correlation between wave heights and water levels within the event and systematically considers the worst possible scenario in terms of coastal flooding. Zachary et al. [2] also examined the approach where one variable is considered of primary

importance and the combination is formed by the maximum of this variable and the concomitant observation of the remaining variable. The results of this statistical approach can vary according to which variable is considered of primary importance. It may severely underestimate the probability of the events where both variables are high, but not maximum, in favor of those with only one variable at its maximum: the consequent joint distribution may inconsistently predict as extremely rare some combinations of (H_s, ζ) that are instead frequently observed in the same time-series from which the pairs are derived.

Recently, by simulating different 100-year scenarios, Lerma et al. [6] showed that the choice of statistical method used to define the forcing conditions for the scenarios produces notable differences in the results.

The appropriate selection approach is reasonably related to the investigated failure mechanism. Hence an alternative procedure consists of generating a single *structure variable* that is a function of the constituent variables (Coles and Tawn [1]). The new variable represents the system response, for instance, the volume of overtopping that is a function of waves and sea levels. This procedure allows a dimension reduction and therefore the use of classical univariate statistics instead of a multivariate extreme value theory (i.e., performed on the constituent variables).

The single *structure variable* method, if accompanied with proper declustering, correctly accounts for correlation within the event but suffers from being site-specific (Zachary et al. [2], Arnoux et al. [7]). In many coastal engineering problems, this is a significant limitation, for instance when dealing with the definition of design conditions for offshore or coastal structures, for which different designs imply different structure variables. Coastal flooding hazard is frequently analyzed with reference to large spatial scales, i.e., an entire coastal region, therefore a site-specific statistical analysis is not a true restraint.

The present study proposes therefore a spatial structure variable, where the term spatial means that it depends on a parameter specific for each area that is subject to flooding. Therefore, according to this approach, the hazard of each storm depends on the area under analysis, so that the combination of water levels and wave height with return period of, say, 100 years, is different for different locations. This makes sense since the critical combination is different for mild and steep shorefaces, high or low dunes, protected and unprotected beaches.

This approach was suggested by a critical reanalysis, in view of two events observed in 2018 and 2019, of the coastal flooding hazard assessment carried out by the Authors (Favaretto et al. [8]) for the Venetian littoral. In fact, due to the selected declustering scheme, the events of combined water levels and wave height observed in 2018 and 2019 appeared extremely rare in the predicted joint extreme statistics.

The objective of this paper is to propose a more reliable methodology to decluster the data, and to apply it to a stretch of coast in the Venetian littoral, which is a low-lying coastal area affected by relatively high storm surges and wind waves that may induce flooding and cause damages to the several high-value touristic cities (Venice, Jesolo, Caorle, etc.) and zones with environmental relevance (Po River Delta). This methodology can be useful to define appropriate mitigation measures and management strategies (Foti et al. [9], Di Risio et al. [10]).

Section 2 presents the declustering scheme, the method to devise the new variable and the sampling procedure. In the third section, the methodology is applied to the Caorle coastline (Venice, Italy) and a comparison of the results of the statistical analysis carried out with the proposed methodology and with more traditional sampling procedures is also presented. Finally, conclusions are drawn.

2. Critical Issues Related to Sea Storm Identification and Sampling

For coastal flooding purposes, many intercorrelated variables can be used to describe sea storms, for instance the significant wave height, the peak wave period, the mean wave direction, the astronomical tide, and the storm surge. In particular, Wahl et al. [11] considered six lamped variables. However, for simplicity, usually only the correlation

between the incident significant wave heights H_s and the offshore sea levels ζ is investigated (for example in [2,3,12,13]).

One of the key aspects to define the joint statistics that arises in the discussion of Coles and Tawn [1] is the importance of defining a sample formed by independent extreme events. Sampling and dependence are closely linked (Mazas and Hamm [14]), therefore, (i) a declustering scheme is necessary to ensure independence and (ii) a criterion for the selection of the parameters that describe the drivers' combination within the cluster (event) is crucial to correctly account for the correlation structure.

2.1. Declustering Scheme for Sea Storm Identification

A *sea storm* is defined as a meteorologically induced disturbance to the local maritime conditions (i.e., waves and water levels) that has the potential to significantly alter the underlying morphology and expose the backshore to waves, currents and/or inundation (Harley [15]).

In order to specify the beginning and the duration of a *sea storm*, a declustering scheme can be used. For statistical convenience, the declustering procedure should identify *independent* sea storms. The identification can in principle be assessed using different variables and different thresholds. Frequently, a meteorological dataset based on wind records is either not available or not easily manageable and the identification involves the analysis of significant wave height H_s and sea-level ζ .

For the purposes of this study, it is considered that *sea storms* are necessarily associated with *wave storms*, and therefore the proposed declustering scheme is only based on the values of H_s . A *wave storm* is defined as a sequence of wave states during which H_s is above a given threshold, named h_{CRIT} . Usually, the threshold h_{CRIT} is related to the average significant wave height $\langle H_s \rangle$ calculated from its time-series, so it depends on the characteristics of the recorded sea states (e.g., $h_{\text{CRIT}} = 1.5 \langle H_s \rangle$, Boccotti, [16]). In order to preserve independency of the data, some technicalities are also considered. It is still the same storm if H_s falls below the threshold but only for a short time interval Δt_{CRIT} (Boccotti, [16], Mendoza et al. [17]), as shown in the central panel of Figure 1. For the choice of Δt_{CRIT} , it is suggested to analyze the autocorrelation function of H_s and find the microscale λ (Taylor [18]) (λ is defined as the intercept of the parabola that matches the curvature of the autocorrelation at the origin with the time axis). For simplicity, it was assumed $\Delta t_{\text{CRIT}} = \lambda$.

Figure 1 shows an example of wind speed (upper panel), significant wave height (central panel) and sea level (lower panel) measurements. The grey boxes highlight two independent *wave storms*, defined by the exceedance of h_{CRIT} , that clearly point out the occurrence of *sea storms* in the wind time series. The sea level ζ is broken down into two sub-processes: the astronomical tide (ζ_A) and the residual (ζ_R). The latter (ζ_R) represents the sum of different contributions (Pasquali et al., [19]), such as storm surge induced by wind and pressure, basin seiching (Bajo et al. [20]), etc. For instance, in the illustrative figure, seiches are evident in the oscillations that appear in the ζ_R series after day 9 and their amplitude fades after 4 days.

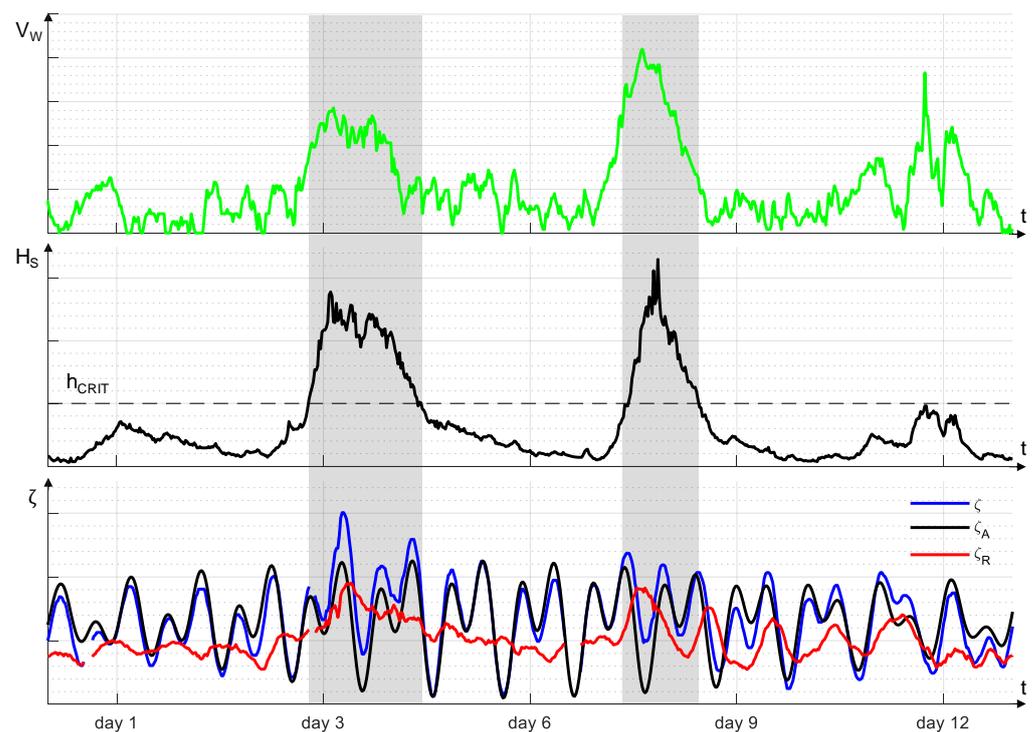


Figure 1. Example of *sea storm* and declustering: the upper panel shows the wind velocity time series (the meteorological forcing), the central panel shows the significant wave height time series and the lower panel shows the sea level time series and its contributions. The grey boxes highlight two independent wave storms, defined by the exceedance of h_{CRIT} .

2.2. Sampling Procedure within Sea Storms

During a *sea storm*, different combinations of simultaneous ζ and H_s can cause flooding. The typical approach would be to select a “*a priori*” combination such as:

- Maximum H_s and simultaneous value of ζ ;
- Maximum ζ and simultaneous value of H_s ;
- Maxima of H_s and ζ during the storm regardless of their concomitance.

These approaches overlook the correlation between the variables or consider one to be of primary importance. A method to overcome this sampling criticism, preserving the correlation between ζ and H_s , consists of defining a third variable function of these drivers. Similar to Mazas and Hamm [2], instead of considering an output variable (i.e., the overtopping discharge), a function that weights the effect of the variables on the flooding can be used for the sampling:

$$r = (\zeta - \langle \zeta \rangle) + a (H_s - \langle H_s \rangle) \quad (1)$$

The selection of the coefficient a is based on the following considerations.

Since the flooding is site-specific and depends on the topography, presence of defense structure, etc., the coefficient should not be uniquely defined, but related to the critical combination of H_s and ζ that cause flooding.

For the evaluation of coastal flooding vulnerability maps, failure is defined in each grid cell of the map when the water depth h_{FLOOD} exceeds a certain value h_{LIM} , which is chosen in this study as 0.2 or 0.5 m, identifying two limit states.

The function that divides the safe region (not flooded) from the unsafe region (flooded) is hence computed through a “limit state” function $g(H_s, \zeta) = 0$ defined as:

$$g(\mathbf{X}) = g(H_s, \zeta) = h_{LIM} - h_{FLOOD}(H_s, \zeta) = 0 \quad (2)$$

The h_{FLOOD} reached in each grid cell of the investigated area can be calculated with a numerical model for a set of pairs of H_s and ζ , representative of the typical range for the site. For the present study, the h_{FLOOD} values are computed with the Simplified Shallow Water (SSW) model developed by Favaretto et al. [21]. This raster-based inundation model solves a simplified form of the Shallow-Water Equations (suited for GPU acceleration) for each cell (pixel) of the domain. Several validations show that this model is capable of simulate wet/dry transitions (Favaretto et al. [22]) and real flood events (Favaretto et al. [21]).

Figure 2 shows an example of the values assumed by $h_{FLOOD}(H_s, \zeta)$ for an illustrative pixel, located on the beach. In the right panel of the figure, dots are the $h_{FLOOD}(H_s, \zeta)$ reached for 90 SSW simulations carried out with different H_s and ζ , the black line is the “limit state” considering a critical value of 0.2 m and the grey area is the unsafe region.

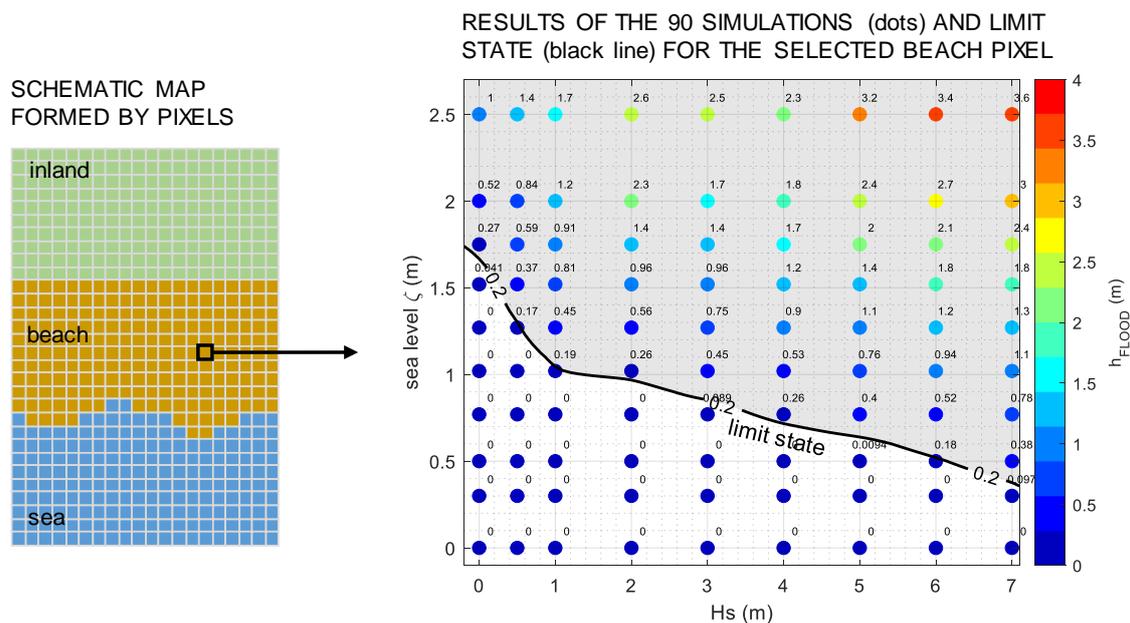


Figure 2. Left: Example of schematic topographic map formed by pixels. Right: Simulated flooding levels (h_{FLOOD}) for a set of H_s and ζ pairs, at the selected beach pixel. The black line is the “limit state” and the grey area is the unsafe region, considering a critical value $h_{LIM} = 0.2$ m.

The coefficient a is reasonably the sensitivity of the limit state to the variable H_s relatively to ζ , and it is found based on the slope of the “limit state” function for each point of the vulnerability map. Actually, the slope should be evaluated at the point that is most likely to induce flooding. This point can be defined exactly once the joint statistics of H_s and ζ is completely known, and it can be assessed with subsequent iterations. In the first iteration, all data points are used without declustering, and the pairs $[H_s, \zeta]$ are considered independent. The proposed procedure follows the First Order Reliability Method (FORM [23]). The variables $[H_s, \zeta]$ are transformed to equivalent independent standard normal random variables $U = (U_H, U_\zeta)$, and the “limit state” function $g(H_s, \zeta)$ is linearized (in similitude with the approach used in Martinelli et al. [24]). The left of Figure 3 shows the standard space of the “limit state” computed with the SSW model and linearized. The point that has the highest failure probability is the one with the shortest distance from the limit state to the origin in the standard space. At this point in the physical space, the slope of the “limit state” (a) is computed (Figure 3 Right). The value r_{LIM} indicates the limit of the structure variable r over which failure occurs.

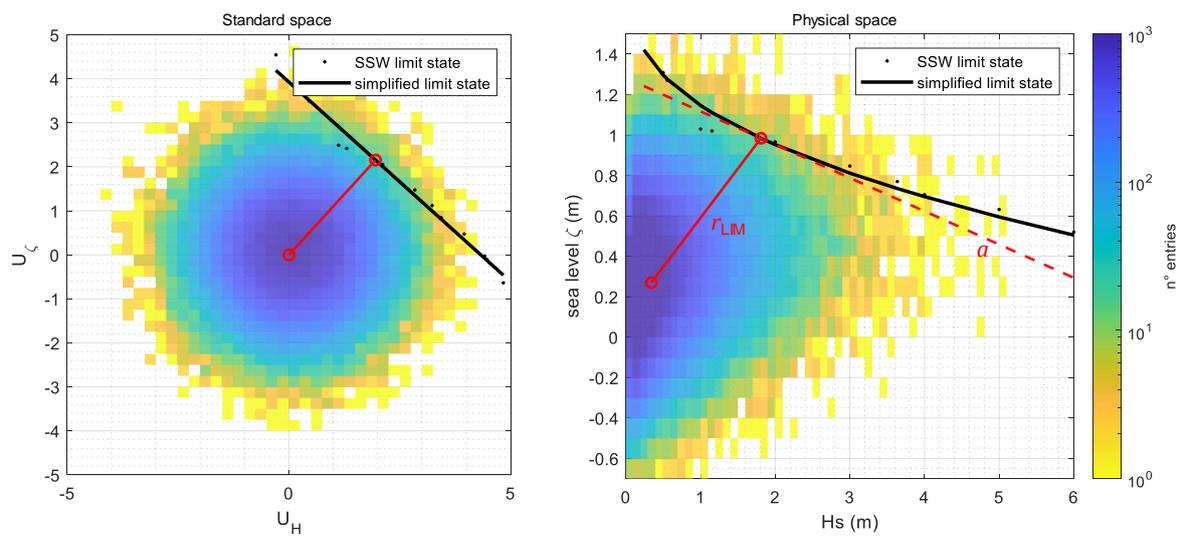


Figure 3. Standard space (left): limit state function calculated with the SSW model (black dots) and linearized. The red segment highlights the point with the shortest distance from the origin. Physical space (right): limit state function derived from back-transformation in the standard space. The dashed line highlights the slope of the “limit space” and the red line is the structure variable limit over which failure occurs. In both spaces also the assumed joint probability density function of U_H and U_ζ and of H_s and ζ is drawn.

The spatial structure variable r is therefore the most likely combination of H_s and ζ that determines the maximum flooding in that particular pixel. During each sea storm, the declustering is carried out identifying the maximum r , as shown in Figures 4 and 5.

The maximum r for each storm provides the critical pairs of H_s and ζ for each location, i.e., for each value of the coefficient a . In theory, these pairs could be used iteratively to define more approximate joint statistics and assuming the actual correlation structure. However, we consider this step unnecessary.

The samples, r , detected in each sea storm, were used to assess a univariate statistical analysis. For simplicity, regions with similar values of a were grouped. Once the univariate variable is defined (Equation (1)), several statistical methods may be applied to carry out extreme value analysis for the marine environment, for instance, Boccotti [16,25], Goda [26] and Laface et al. [27]. The Generalized Pareto Distribution (GPD) was chosen, being suitable to describe a sample of independent, identically distributed random variables conditioned by a threshold. The Maximum Likelihood (ML) estimation procedure was used to select the probability density function parameters (the shape and scale parameters). The choice of the threshold can be based on two empirical techniques (Coles et al. [28]): one is based on the interpretation of the mean excess (ME) plot, which should be approximately linear in the proximity of the appropriate threshold, and the other is based on the stability of shape and scale parameter of GPD in the vicinity of the threshold. The latter method was used, trying to maintain a low threshold where possible.

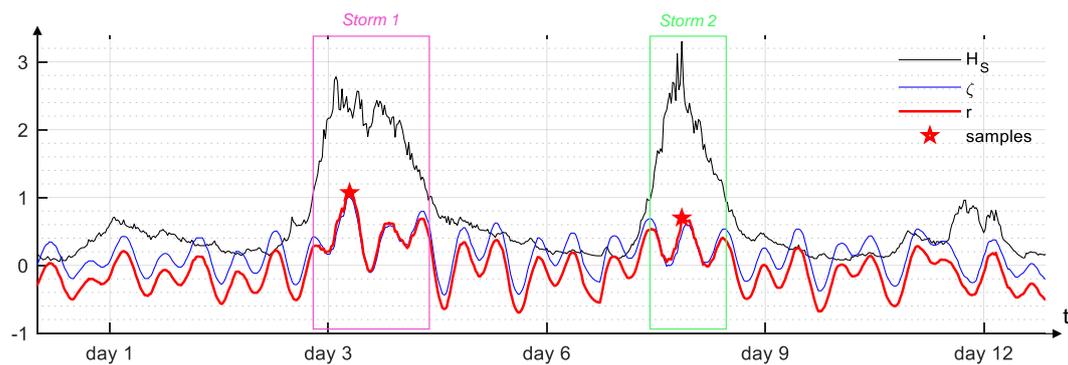


Figure 4. Choice of the samples for each sea storm. The red line is the structure variable determined for an illustrative pixel and used for the sampling. The red stars are the chosen samples.

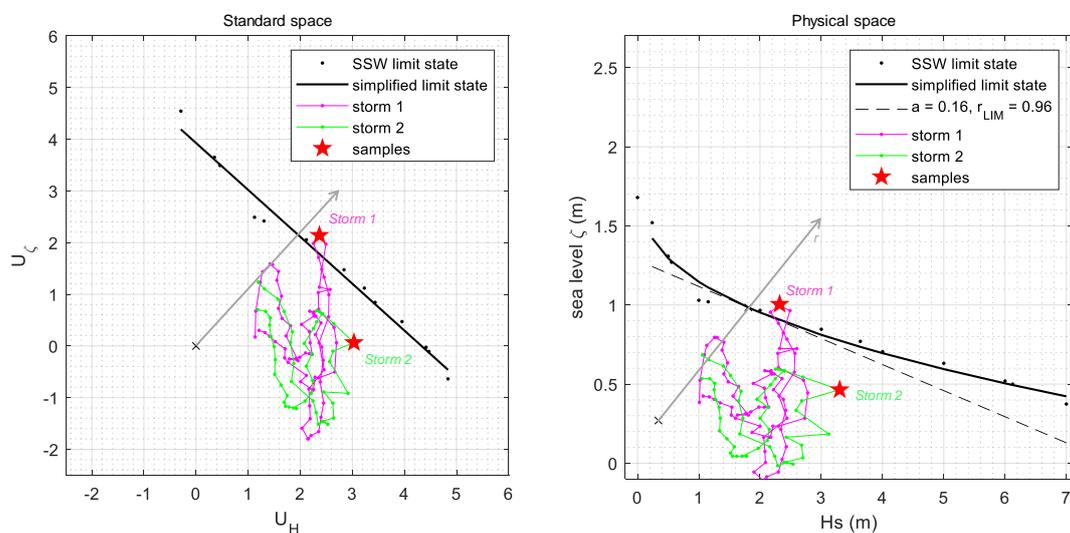


Figure 5. Example of how storms 1 and 2, identified in Figure 4, appear for a specific pixel: (left) in the standard space and (right) back-transformed in the physical space.

3. Case Study

The Venetian littoral faces the North Adriatic Sea and is characterized by two main wind (and correspondingly wave) regimes, Bora and Scirocco, which blow from the Northeast and Southeast, respectively (Umgiesser et al. [29]). Waves up to 6 m and high storm surges are responsible for the flooding of this coastal area as well as Venice.

Recently, two extreme events occurred along the Venetian littoral in 2018 (Vaia storm described in [30,31]) and in 2019 ([32,33]). The first event was characterized by an extreme significant wave height of 5.92 m and a sea level of 1.56 m relative to the local tidal reference (named ZMPS, Zero Mareografico Punta Salute), conversely, the 2019 event was characterized by an extreme sea level of 1.82 m ZMPS and a wave height of 3.5 m. Both events caused localized erosion and marine ingression along the Venetian coast.

Caorle is a coastal town located on the Northeast side of the Venetian littoral and is one of the 20 coastal cells identified by Ruol et al. [34]. The Caorle coastline is 5 km long and is confined between the lagoon's inlet of Falconera to the North and the mouth of the Livenza river to the South, both protected with jetties (Figure 6). The cell can be subdivided into three main parts: (i) "Spiaggia di Levante" to the Northeast; (ii) "Murazzi" in the central part; and (iii) "Spiaggia di Ponente" to the Southwest. The historic town is in the central part, protected by a sea wall that stabilizes the shoreline and mitigates the risk of flooding. North of this sea wall, there is a cusp, where the historic church named "Chiesa della Madonna dell'Angelo" is situated. The entire cell is urbanized and no system of dunes

is present. The economy is mainly based on tourism ($\sim 4,300,000$ visitors in 2018, i.e., the 9th city in Italy for the number of presences) and fishing activities. Natural subsidence affects the area, with a rate of -2 to -4 mm/year [35].

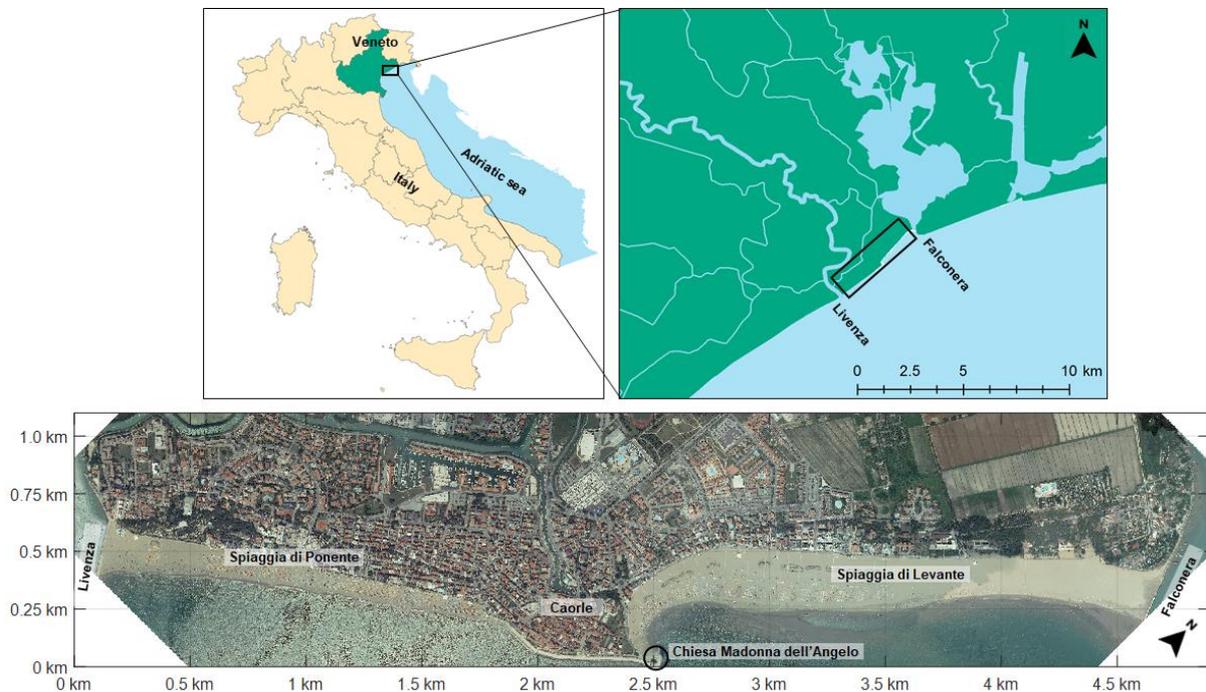


Figure 6. Location and orthophoto of the study site: the Caorle coastline.

Available Data

For the area under investigation, data and information were collected, harmonized and stored in a Coastal GIS [34]. The topographic data, useful for the present study, comprises Digital Elevation Model (DEM, 1 m resolution) from a Lidar survey (2013) and bathymetric surveys (2014).

Times series of sea levels and significant wave heights are available at the oceanographic tower “Acqua Alta” situated 8 miles offshore in a water depth of 16 m in the Gulf of Venice (Lat $45^{\circ}18'51.27''$ N, Lon: $12^{\circ}30'29.93''$ E). The H_s time series is available from October 1987 to December 2007 with a recording time step Δt of 3 h and from January 2008 to March 2017 with a $\Delta t = 30$ min. The sea levels are available from January 1980 to December 2018 with a $\Delta t = 1$ h. For the 2018 event, the H_s is available during October ([31]) every 1 h. For the 2019 event (from 1 November to 1 December 2019), the H_s and ζ are available every 10 min and measured by the “Centro Previsioni e Segnalazioni Maree—CPSM, Venice” institution. To get a homogeneous bivariate series, the H_s dataset is firstly interpolated every 3 h and then the time step is re-set equal to 1 h as for the ζ dataset. Only the simultaneous recording period is taken into account, i.e., from October 1987 to March 2017. The two extreme events (2018 and 2019) are included in the statistical analysis and their representative values of H_s and ζ are taken every 1 h in accordance with the 1987–2017 dataset.

The annual mean sea levels ζ_{MSL} for the Venetian littoral [36] are available from 1870 to 2017 measured at “Punta della Salute” station, situated approximately in front of the St. Mark square in the city of Venice. The ζ_{MSL} is the long-term trend, described as a function of time, whereas ζ^* (defined as $\zeta^* = \zeta - \zeta_{MSL}$) is a stationary stochastic variable.

4. Results

Following the proposed methodology, a dataset of 90 flooding maps is assessed, considering an offshore significant wave height H_s ranging from 0 m to 7 m and ζ^* ranging from 0 m to 2.5 m.

The SSW model is used to evaluate each map. The grid is formed by regular cells ($dx = dy = 1$ m) and covers an area of ~ 1.5 km \times 4.9 km (7.5×10^6 cells); the Gauckler–Strickler coefficient K_s used is $50 \text{ m}^{1/3}/\text{s}$. Two different boundary conditions were considered: one at the shoreline, i.e., sea levels and significant wave heights, and the other at the two mouths (*Livenza* and *Falconera*), i.e., only the sea levels. Each simulation covers only 6 h and the flooding maps include the maximum flooded depth reached during the simulations.

These resulting maps were used to evaluate the coefficient a and the r_{LIM} value of the structure variable, considering $h_{\text{LIM}} = 0.2$ m, i.e., a “nuisance flooding” (Moftakhari et al. [37]). This type of flooding refers to low levels of inundation that do not cause notable threats to people or extensive damages, but it can disturb daily activities, add strain on infrastructures (roads, sewers, drainage systems, etc.) and cause minor damages to private properties.

Figure 7 shows the distribution of the coefficient a in the Caorle area. Four areas can be detected: (i) $a = 0.05$ (blue): inland area with elevation above 0 m; (ii) $a = 0.10$ (light blue): inland area with elevation below 0 m; (iii) $a = 0.15$ (yellow): emerged beach and inland next to the river mouths; (iv) $a = 0.20$ (red): emerged beach.

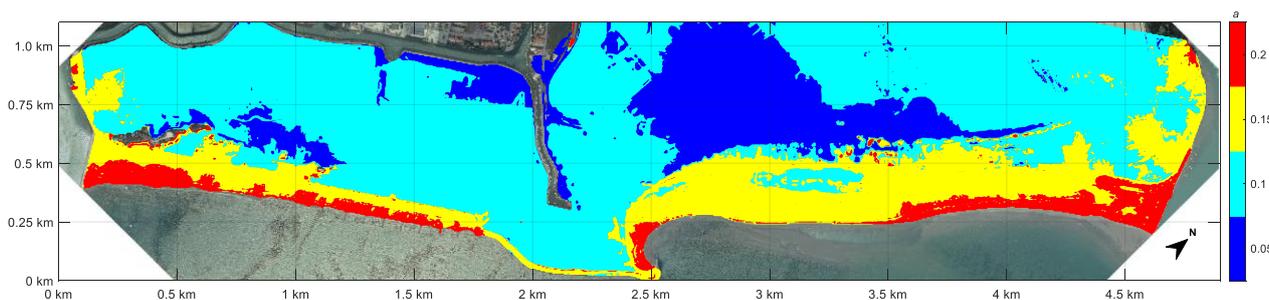


Figure 7. Distribution of the coefficient a in the investigated area.

Figure 8 shows the distribution of the r_{LIM} for the areas with $a = 0.12$ to 0.17 , corresponding to the yellow area in Figure 7.

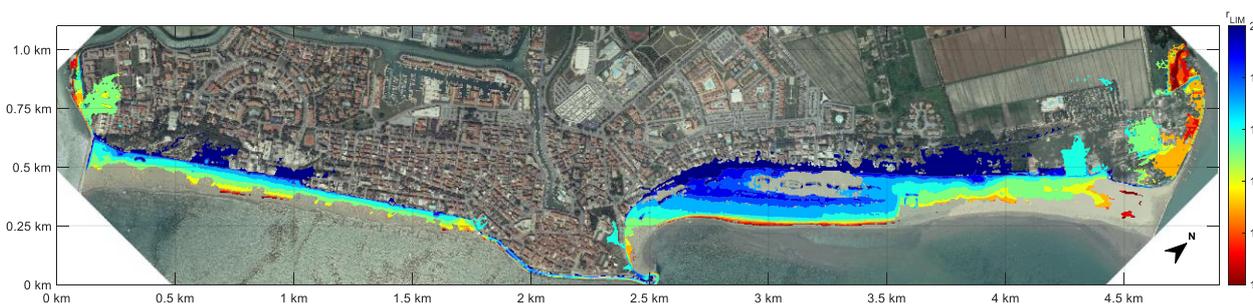


Figure 8. Distribution of the r_{LIM} for the zones with $a = 0.12$ to 0.17 .

The time series of H_s and ζ^* (introduced above) covers 31.5 years. From the time series of H_s , following the methodology described above, the storm identification is performed considering h_{CRIT} equal to 1 m and selecting Δt_{CRIT} equal to 12 h from the analysis of the autocorrelation function. The total number of independent storms identified is 1057, i.e., ~ 34 events per year. Four samples r can be derived, corresponding to the different zones highlighted in Figure 7, in order to establish coastal hazard maps.

The samples, r , are analyzed with a univariate approach, fitting the data to a Generalized Pareto Distribution (GPD). The MLE procedure is used to select the probability

density function parameters. The threshold is selected in order to have the same number of extremes for each coefficient a .

The evaluation of the four statistical analyses allows to assess the hazard map through the evaluation of the probability $P\{r > r_{LIM}\}$, i.e., in this case the probability that a flooding level equal to 0.2 m is exceeded. Figure 9 shows the hazard map for the Caorle area, expressed as a function of the return period T_R .

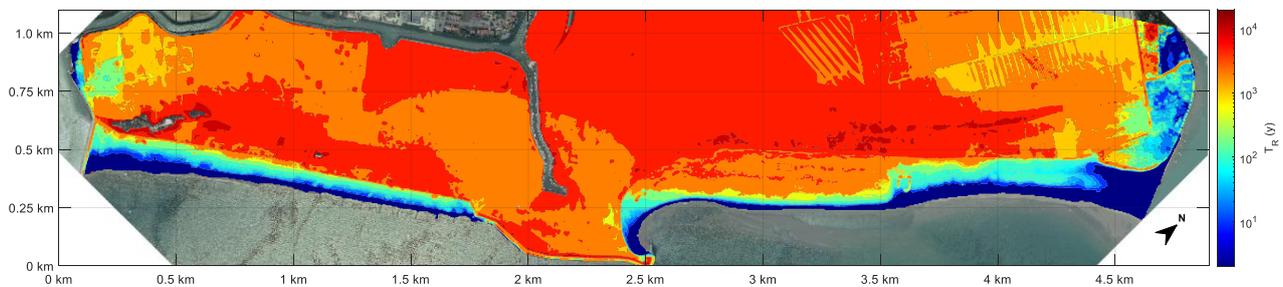


Figure 9. Hazard map for the Caorle coastline, i.e., the probability that a flooding level 0.2 m is exceeded, expressed as a function of the return period.

Discussion on the October 2018 and November 2019 Events

The stormy event that occurred on 28 October 2018 developed as a consequence of an explosive cyclogenesis in the Western Mediterranean Sea (Cavaleri et al. [31]). This led to intense winds blowing from the Southeast (Scirocco) that caused severe damages in the Dolomites area. Extreme waves and high-water levels were originated in the Northern Adriatic Sea, and the recorded significant wave height was up to 6 m.

On 12 November 2019, a severe stormy event occurred in the Venetian littoral, characterized by high waves (H_s up to 3.5 m) and an extreme sea level. The water level reached 1.82 m ZMPS at 22.50 UTC (measured at the oceanographic tower), the second-highest value ever measured in this area. This level was caused by a combination of an astronomical tidal peak with a severe storm surge generated by a strong wind (up to 30 m/s) and a sudden pressure drop down to 987 hPa. The wind direction quickly turned from 100° N to 230° N. During this event, several failures occurred along the Venetian littoral. Venice city was completely flooded, causing damages to historical heritage, houses, and boats. The Venetian coast was affected by localized erosions and even by coastal flooding phenomena that caused damages to many touristic and productive facilities.

The left of Figure 10 shows the sea levels and the significant wave heights measured at the CNR oceanographic tower during these two extreme events. Two instants are highlighted with stars that indicate the chosen sample for an area with a equal to 0.17. The right of Figure 10 shows the development of the two events in the H_s - ζ plane and the limit state for a pixel characterized by $a = 0.17$ and $r_{LIM} = 1.1$ m. The two events caused the failure since both exceeded the limit state.

The statistical analysis carried out on the structure variables, built with different a values and presented in the previous paragraph, allows for estimating the return period of the two events. The results are summarized in Table 1 in terms of chosen couples, values of the structure variable for the events (r_0), exceedance probability $P(r > r_0)$, and return period T_R (where λ is the number of events per year). Table 1 shows also, in the last three rows, the results carried out with three more traditional sampling procedures. In detail, the following samples were taken into account: (S1) Maximum H_s and simultaneous value of ζ^* ; (S2) Maximum ζ^* and simultaneous value of H_s ; (S3) Maxima of H_s and ζ^* during the storm regardless of their concomitance. For these samples, copulas are used to describe the dependence structure associated with the joint distribution of the two variables. A thorough introduction to Copula modelling and a large selection of the most common families are provided in [38,39]. Similar to the analysis of De Michele et al. [40], the Gumbel–Hougaard distribution, i.e., an Archimedean extreme value copula, is selected

as the best candidate to model the data. To estimate the model parameters θ , the maximum likelihood estimation method (MLE) guarantees that the dataset is the most probable under the assumed statistical model. The 2018 event was characterized by extreme waves and high sea levels; conversely, the 2019 event was characterized by extreme sea levels and high waves. For the 2018 event, the return period (T_R) based on sample $S1$ is very high. Analyzing the same event considering sample $S2$, the associated T_R is low and the exceptionality is not confirmed. For the 2019 event, the T_R based on sample $S2$ is very high and the T_R based on sample $S1$ low. Both events appear to be very rare with the analysis on sample $S3$, assuming that the maxima occur at the same time in the event.

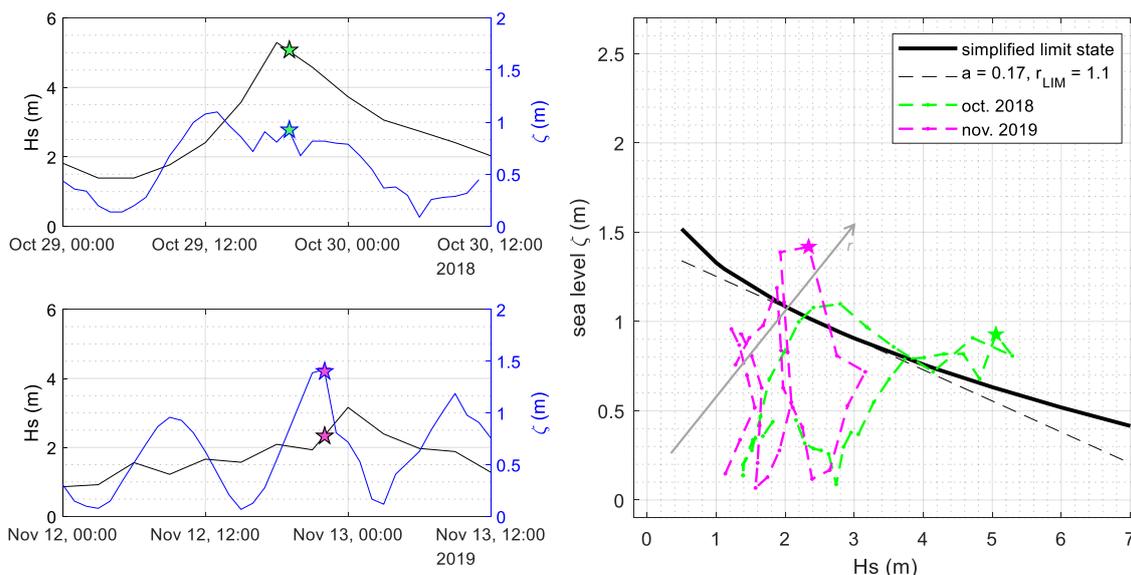


Figure 10. Measurements of the 28 October 2018 and 12 November 2019 stormy events (left and right) and their evolution in the H_s - ζ plane.

Table 1. Return periods for the two extreme events occurred along the Venetian littoral in 2018 and 2019 evaluated considering different types of samples.

Samples	October 2018					November 2019				
	H_s (m)	ζ^* (m)	r_0 (m)	$P(r > r_0)$	T_R (y) $=1/[\lambda P(r > r_0)]$	H_s (m)	ζ^* (m)	r_0 (m)	$P(r > r_0)$	T_R (y) $=1/[\lambda P(r > r_0)]$
$a = 0$	2.80	1.10	0.83	0.0887	6	2.34	1.42	1.15	0.0029	181
$a = 0.05$	2.80	1.10	0.95	0.0770	7	2.34	1.42	1.25	0.0031	166
$a = 0.1$	5.05	0.93	1.13	0.0455	11	2.34	1.42	1.35	0.0053	97
$a = 0.15$	5.05	0.93	1.36	0.0217	24	2.34	1.42	1.45	0.0109	47
$a = 0.2$	5.05	0.93	1.60	0.0125	41	2.34	1.42	1.55	0.0189	27
$a = 0.25$	5.05	0.93	1.84	0.0079	66	2.34	1.42	1.65	0.0317	16
$a = 0.3$	5.05	0.93	2.07	0.0078	66	2.34	1.42	1.75	0.0493	10
$a = 0.35$	5.05	0.93	2.31	0.0084	62	2.34	1.42	1.85	0.0681	8
$a = 0.4$	5.05	0.93	2.54	0.0108	48	2.34	1.42	1.95	0.0865	6
$a = 0.45$	5.05	0.93	2.78	0.0096	54	2.34	1.42	2.05	0.1093	5
$a = 0.5$	5.05	0.93	3.01	0.0110	47	2.34	1.42	2.15	0.1304	4
S1: Max H_s and simultaneous ζ	5.29	0.81	-	0.0026	258	3.16	0.72	-	0.1020	7
S2: Max ζ and simultaneous H_s	0.72	1.10	-	0.0377	13	2.34	1.42	-	0.0050	102
S3: Maxima of H_s and ζ	5.29	1.10	-	0.0024	91	3.16	1.42	-	0.0011	197

From the hazard map presented in Figure 9, the point $x = 2.5$ km, $y = 0.3$ km, i.e., the one corresponding to picture no. 2 of Figure 11, has $a = 0.15$ and $r_{LIM} = 1.4$ m. From Table 1, the r_0 of November 2019 is larger and therefore flooding is expected. The associated return period in this area is $T_R = 47$ years. In fact, marine ingression occurred along the Caorle coastline during November 2019: both the beaches were completely flooded and the waves reached the church at the cusp (pictures in Figure 11).

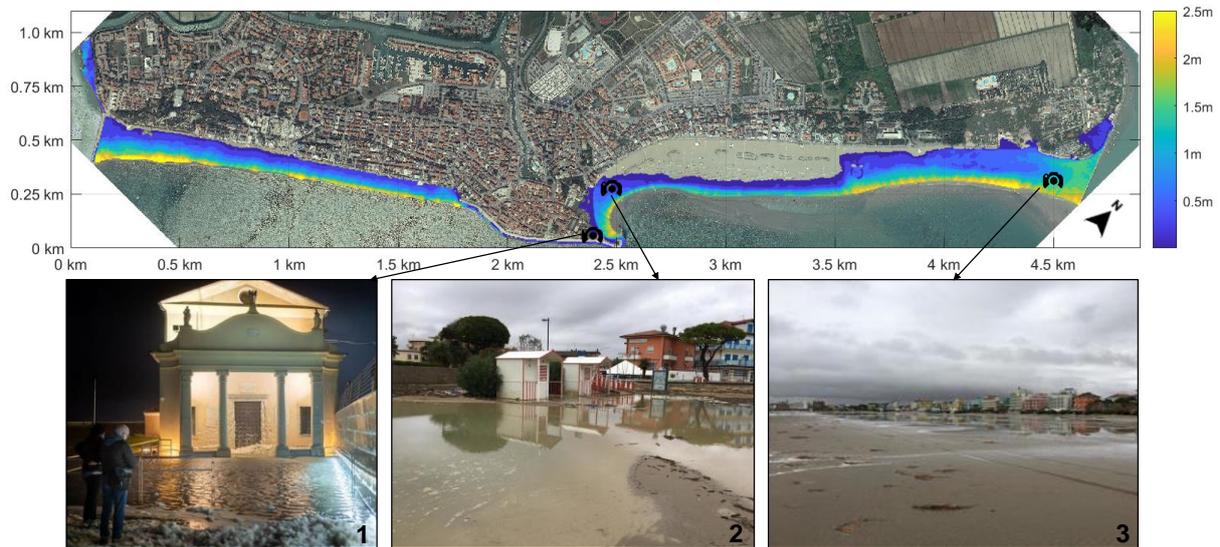


Figure 11. SSW simulation results for the November 2019 event and photographs taken the day after.

The SSW model is used to simulate this event and it is initialized with the same grid and the same conditions described before. The simulation covers only 4 h around the peak of sea level. Figure 11 shows the maximum flooded level reached during the simulation. Since the flooding occurred during nighttime, only some qualitative information and some photographs, taken the morning after, are available (Figure 11). However, these photographs show the damages and the residual flooding on the beach and at the church. The simulation results are consistent with this information.

5. Conclusions

In many low-lying coastal regions, flooding hazard assessment is critically affected by the correct evaluation of the simultaneous occurrence of the significant wave height H_s and the water level ζ . In turn, the bivariate statistics is significantly affected by the methodology used for the selection of the independent sample pair H_s, ζ during a sea storm.

Following Mazas and Hamm [2], the suggested sampling procedure is to define a *structure variable* that considers the combination of H_s and ζ that is the most critical for flooding. This study proposes a procedure to find a spatial structure variable, that is a linear combination of water level and wave height, specific for each location, based on the slope of the failure function at the design point.

As a consequence, the statistical analysis is specific to each point of the area at risk. For instance, for low-lying points located far from a coastal dune system, ζ is more important than H_s , and hence the relative flooding occurrence is mainly affected by the maximum water levels statistics. Conversely, for an area located just behind a seawall where overtopping is relevant, the role played by H_s is dominant, and the relative flooding occurrence is strongly affected by the maximum significant wave height statistics.

The proposed procedure was applied to a stretch of the Venetian littoral that was affected by marine ingression during two recent stormy events (occurred in 2018 and 2019). A map showing the chosen combination of the variables is presented together with the hazard map for the investigated coastline.

The paper draws attention to the (apparently trivial) need for a consistent procedure for sampling multivariate events from sea measurements. Based on real examples, it is shown that the method used to sample the bivariate event formed by H_s and ζ significantly affects the return period of the critical storm and the sampling method should therefore be considered as an important aspect for the evaluation of the flooding hazard. The comparison between traditional sampling methods and the proposed one highlights that the latter gives more consistent results in terms of return periods.

The method can be implemented to coastal stretches subject to rather uniform environmental conditions. For instance, for the Venetian littorals, it was assessed that the length of each stretch is of order 3–12 km.

As further work, it is envisaged that application of the procedure to many real and schematic cases could point out some a priori guidelines to select the proper combination of water level and significant wave height only based on the morphological characteristics of the site, thus defining a priori the proper structure variable for the location of interest.

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