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Influence of Combined Action of Steel Fiber and MgO on Chloride Diffusion Resistance of Concrete

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Received: 24 March 2020; Accepted: 22 April 2020; Published: 24 April 2020



Abstract: To improve the chloride diffusion resistance and durability of concrete, a new kind of steel fiber reinforced MgO concrete (SFRMC) was made by adding steel fiber and MgO to concrete simultaneously. With steel fiber for load bearing and expansion limiting, MgO as the expander, SFRMC has both the advantages of fiber reinforced concrete and expansion concrete. The influence of steel fiber and MgO on the strength and chloride diffusion resistance of concrete was evaluated by splitting tensile test and chloride diffusion test. Mercury intrusion porosimeter (MIP) and scanning electron microscopy (SEM) were used to study the microstructure of SFRMC. The results showed that the combined action of steel fiber and MgO reduced the porosity of concrete and the chloride diffusion coefficient (CDC), which could not be achieved by steel fiber and MgO separately. In the free state, the expansion energy produced by the hydration of MgO made the concrete expand outwards. However, under the constraint of steel fiber, the expansion energy was used to tension the fiber, resulting in self-stress. In this way, compared to reference concrete RC, the tensile strength of SFRMC-1, SFRMC-2, and SFRMC-3 increased by 3.1%, 61.3%, and 64.5%, CDC decreased by 8.8%, 36.7%, and 33.1%, and the porosity decreased by 6.2%, 18.4%, and 20.6%, respectively. In addition, the SEM observations demonstrated that the interfacial transition zone (ITZ) between fiber and matrix was denser in SFRMC, which contributed to reduce the diffusion of chloride ions in the concrete.

Keywords: steel fiber; MgO expansive agent; split tensile strength; chloride diffusion resistance; porosity; interfacial transition zone

1. Introduction

Steel fiber reinforced concrete (SFRC) is a kind of high-performance concrete which distributes steel fiber uniformly in the matrix. Due to the restraint of steel fibers, the development of cracks is restrained, which makes SFRC have higher crack resistance and good toughness [1]. Owing to these excellent properties, SFRC has been widely used in port structures and protection structures, especially in those structures with high requirements for diffusion resistance and crack resistance [2–4]. However, these structures mentioned above are often in direct contact with chlorine ions. When chloride ions diffuse into the concrete, they will cause corrosion of the steel bar, leading to cracking and affecting the safety of the structure [5,6]. Therefore, it is of great significance to reduce the chloride diffusion coefficient (CDC) of SFRC.

In recent years; many researchers have studied the chloride diffusion resistance of fiber reinforced concrete. Vahid compared the effects of different kinds of fibers on the resistance of concrete to chloride



tolerance. He found that polypropylene fiber reduced CDC; while adding steel fibers significantly increased CDC [7]. Guo added 0.15%, 0.30%, 0.45%, 0.60% of basalt fiber into concrete and tested the pore structure and the chloride diffusion resistance. The results showed that by adding basalt fiber, the chloride diffusion resistance was improved, the minor harmful pores (20–100 nm) were increased, and the serious harmful pores (>200 nm) were significantly reduced [8]. Yan explored the effects of different amounts of basalt fiber (BF; 0.05, 0.1, 0.15, and 0.2 vol%) on the chloride ion diffusion. He found that the inclusion of 0.05% BF accelerated the diffusion of chloride ions from the coral aggregate; while a dosage of BF above 0.1% could suppress the diffusion of chloride ions [9]. Mahyuddin explored the effects of different amounts of coconut fiber (0.6%, 1.2%, 1.8%, and 2.4%) on mechanical property and chloride diffusion. He found that by adding coconut fiber, compressive and flexural strengths were increased to 13% and 9%, respectively. However, in terms of durability, the chloride diffusion resistance was reduced [10]

Previous studies have confirmed that fiber can improve the mechanical properties of concrete [11–16], but there are different opinions on the chloride diffusion resistance. Some researchers think that fiber can enhance the chloride diffusion resistance of concrete, but the other researchers hold the opposite opinion. The main reason for different opinions is that fiber has both positive and negative effects in concrete. On the one hand, fiber reduces the shrinkage crack of concrete and improves the compactness of concrete. On the other hand, the fiber increases the number of ITZ between the fiber and the matrix, and the gap at the interface provides a channel for chloride diffusion. However, researchers have agreed that ITZ between fiber and matrix is the weakness of concrete, and the strength of ITZ has a great influence on the performance of concrete. To reduce the defect of ITZ, steel fiber and MgO were used simultaneously in this paper. The expansion of MgO was restrained by steel fiber, the void was filled, the porosity was reduced, and the strength of ITZ was improved.

From the view of reducing shrinkage cracks, the use of expansion agent is to use the expansion produced by hydration to compensate the shrinkage of concrete [17]. According to the differences in mineral composition of the expansive agent, the expansive agents currently sold in the market can be divided into five types: sulfide-aluminate expansive agent, lime expansive agent, iron powder expansive agent, MgO expansive agent, and compound expansive agent [18]. Among the above, MgO is one of the most excellent expansive agents. It has been widely used because of its stable hydration products, easy regulation of expansion performance, and no shrinkage in the later stage [19,20].

By studying the advantages and disadvantages of fiber and MgO, we found that steel fiber and MgO can be used together to give full play to the advantages of both materials. Different from SFRC, steel fiber not only plays a role in bridge connection, but also plays a role in limiting expansion in SFRMC. The three-dimensional distribution of steel fiber seriously restricts the expansion of MgO, changes the expansion mode of MgO, and makes the expansion from outward extension to inward extrusion, which greatly improves the compactness of SFRMC. Our previous research [21] has confirmed that SFRMC has excellent mechanical properties, but its durability has not been studied. In this paper, split tensile test, chloride diffusion test, mercury intrusion porosimeter MIP test, and scanning electron microscopy SEM test were conducted to study the combined effect of steel fiber and MgO on the tensile strength and chloride diffusion resistance of SFRMC. Through the analysis of pore structure, ITZ structure, the mechanism of SFRMC performance enhancement was studied. These data provided theoretical support for further application of SFRMC and opened up new application fields for MgO concrete and fiber reinforced concrete.

2. Materials and Methods

2.1. Materials

In this paper, the cement is Class 52.5 Ordinary Portland Cement (Shan Aluminum Cement Co., Ltd., Shandong, China). Fly ash was produced by Shenhua Huashou Power Co., Ltd in Shanghai, China. MgO was produced by Wuhan Sanyuan Special Building Materials co. LTD in Wuhan, China.

The activity of MgO was 115 s and the specific surface area was 45.7 m^2/g . Figure 1 shows the mineral composition of MgO. Table 1 summarizes the chemical composition of cement, Fly ash, and MgO.



Figure 1. X-ray diffraction pattern of MgO.

Table 1.	Chemical	composition	of	cement.

Type	Chemical Composition /wt%										
Type	CaO	MgO	Al_2O_3	SiO ₂	Fe ₂ O ₃	SO_3	K ₂ O	Na ₂ O	Loss	Total	
Cement	60.51	2.18	6.34	22.02	3.05	1.86	0.47	0.23	1.96	98.62	
Fly ash	5.01	1.03	34.18	48.91	5.22	1.20	0.89	0.62	1.50	98.56	
MgO	3.19	85.44	0.73	4.45	0.42	0	0	0	4.49	98.72	

Continuously graded gravel with a size of 5–20 mm was used as the coarse aggregates. River sand with a fineness modulus of 2.94 was used as the fine aggregates. Steel fiber was produced by Zibo Shuanglian Building Materials co. LTD in China. The tensile strength of steel fiber was 520 MPa. The diameter of steel fiber was 0.58 mm and the length was 40 mm. Figure 2 shows the specific morphology of steel fiber, which is a wave shape. Table 2 shows the mix proportion of the concrete used in this paper.



Figure 2. Photograph of steel fiber.

2.2. Experimental

The specific process and analysis method of this paper are shown in Figure 3. Cube blocks of 150*150*150 mm were poured to study the split tensile strength and cylinder specimens of $\emptyset 100 \times 50$ mm were poured to study the chloride diffusion resistance. After casting, the molds filled with fresh concrete were placed on the high-frequency vibration table to vibrate until no obvious bubbles escaped. After 24 h, the abrasives were removed and the concrete was transferred to the standard curing chamber ($20 \pm 2 \,^{\circ}C$, 95% RH).

	C1	Composition /kg·m ⁻³									
Specimen	mm	Cement	Fly Ash	Fine Aggregate	Coarse Aggregate	Water	Water Reducer	Steel Fiber	MgO		
Ref (RC)	155	450	50	713	1025	160	6	0	0		
8%MgO(MC)	132	450	50	713	1025	160	6	0	40		
0.5%Fiber(SFRC)	141	450	50	713	1025	160	6	39	0		
0.5%Fiber + 8%MgO (SFRMC-1)	128	450	50	713	1025	160	6	39	40		
1%Fiber+8%MgO (SFRMC-2)	115	450	50	713	1025	160	6	78	40		
1.5%Fiber + 8%MgO (SFRMC-3)	103	450	50	713	1025	160	6	117	40		

Table 2. Mix proportion of concrete.



Figure 3. The specific process and analysis method.

2.2.1. Split Tensile Test

The split tensile strength test was conducted with reference to China Standard GB/T 50081-2016 [22]. The main equipment was the SYE-2000 pressure testing machine with a maximum load of 2000 kN. The size of the specimens was 150 mm \times 150 mm \times 150 mm, and the mean value of the three specimens was taken as the tensile strength of the specimens (accurate to 0.01 Mpa). The loading rate was 0.06 Mpa/s. The tensile strength was calculated according to Formula (1). The test diagram is shown in the Figure 4:

$$f_{ts} = 2F/\pi A = 0.637F/A,$$
 (1)

where f_{ts} is the tensile strength of concrete, in MPa; *F* is the failure load of the test piece, in N; *A* is the bearing area of the test piece, in mm².

2.2.2. Chloride Diffusion Test

Normally, the main methods for measuring chloride diffusion resistance are slow method and fast method. Among them, the slow method is too time-consuming and seldom used in practice, while the fast method can measure the chloride diffusion resistance of materials in a short time. As the most typical of the rapid method, the electric flux method was first proposed by the Portland Cement Association of the United States in 1981 and has become the most widely used in the world. The principle of the method is to use the electric field to accelerate the ion transport. The ions penetrate through the specimen under the action of DC power. In this paper, the chloride diffusion resistance



was evaluated by the flux transferred in accordance with the procedures described in ASTM C1202 [23]. The test device is shown in Figure 5.

Figure 4. Setup for split tensile strength test: (a) Schematic diagram of test; (b) Photo of test.



Figure 5. Setup for the chloride diffusion test.

Cylindrical specimens with a diameter of 100 mm and a height of 50 mm were used in the test. The solution was 0.3 *mol/l* NaOH and 3% NaCl. SX-DTL concrete chloride flux meter was used to record the electrification time and flux. The total electric flux of the concrete test block for 6 h was calculated according to Formula (2):

$$Q = 900 \times (I_0 + 2I_{30} + 2I_{60} + \dots + 2I_t + \dots + 2I_{300} + 2I_{330} + I_{360}),$$
(2)

where *Q* is total flux through test block for 6 h (C); I_0 is initial flux (A), to 0.001A; I_t is flux (A) at time *t* (min), to 0.001A.

According to the Nernst–Plank equation, the relationship between chloride diffusion coefficient (CDC) and Q was established (Formula (3)):

$$CDC = 2.57765 + 0.00492 \times Q.$$
(3)

2.2.3. MIP Test

The pore structure of concrete was analyzed by mercury intrusion porosimetry (MIP). When making the specimen, the concrete was knocked into several small test blocks with a length of about 2 mm to remove the coarse aggregate. The specimens were then soaked in anhydrous ethanol for 24 h to stop the hydration of cement and MgO. Then, the specimens were placed in a vacuum drying oven at 50 °C for drying for 12 h. Finally, the dried specimens were sealed in a plastic bag to prevent moisture from entering. During the test, the pore structure of two or three specimens was tested by MIP, and the effect of steel fibers and MgO on the pore structure was observed. By analyzing the experimental data of the pores, we could explain the variation law of tensile strength and compressive strength from the perspective of microstructure.

2.2.4. SEM Test

SEM test (JSM-6510lA, Japan) was used to study the interface performance between steel fiber and substrate. All specimens used for SEM test were standardly cured for 28 days.

3. Results

3.1. Failure Pattern of Concrete

At the beginning of the split tensile test, the specimen was in the elastic stage, the surface was intact, and the deformation increased proportionally with the increase of load. With the continuous increase of the load, the vertical cracks appeared in the middle of the specimen, and then gradually extended to the bottom and the top, accompanied by the sound of splitting. At the end of test, the specimen was completely damaged. There were no obvious signs before the failure of RC and MC. After the cracks appeared on the surface, they extended rapidly and destroyed the concrete completely, showing obvious brittle failure. Finally, the specimen broke into two independent parts (Figure 6a,b). When the steel fiber was added into concrete, owing to the steel fiber limited the crack extension, the crack developed slowly before the destruction of SFRC, showing obvious ductility. Some fine cracks appeared around the long crack, and the specimen remained as a complete whole after the test, with only a small amount of block concrete falling from the surface (Figure 6c).



Figure 6. Failure pattern of concrete. (**a**) Reference concrete (RC); (**b**) MgO concrete (MC); (**c**) Steel fiber reinforced concrete (SFRC); (**d**) Steel fiber reinforced MgO concrete (SFRMC).

On the other hand, when the steel fiber and MgO were used simultaneously, the steel fiber restrained the expansion of MgO and generated self-stress, which improved the bite force between steel fiber and matrix, enhanced the strength of the interface between fiber and matrix, and significantly enhanced the crack resistance. Steel fiber made cracks became more evenly distributed. Instead of a single long and wide main crack in RC, the crack developed into many parallel fine cracks in SFRMC.

After the test, the SFRMC surface was smooth without concrete spalling, and the specimen split but did not separate (Figure 6d).

3.2. Combined Action of Steel Fiber and MgO on Split Tensile Strength

Figure 7 shows the split tensile strength of specimens with different mix proportions. Compared to RC, MgO had a small impact on the strength of MC in the early stage, but it had a greater impact on the strength in the later stage. At 28 days, the tensile strength of MC was 4.4% higher than that of RC, and the improvement was mainly due to the hydration of MgO. Compared to RC, the tensile strength of SFRC had been significantly improved. The strength had increased by 29.7% at 28 days, which was the result of the steel fiber restrained crack extension.



Figure 7. Split tensile strength of concrete.

On the other hand, when MgO and steel fiber were used at the same time, the tensile strength of SFRMC continued to increase. Compared to RC, the tensile strength of SFRMC-1, SFRMC-2, and SFRMC increased by 3.1%, 61.3%, and 64.5%, respectively. The tensile strength increased with the increase of steel fibers, and the growth rate was greater when the fiber was less than 1%, and it would no longer increase significantly when it exceeded 1%. Therefore, considering the cost of materials, we recommend using 8% MgO and 1% steel fiber in constructions.

As we can see from the split tensile test, the concrete strength had been significantly improved under the combined action of steel fiber and MgO. There are two main reasons for the increase. The first is that steel fiber limited the crack extension. When the cracks came out, the tensile stress was transferred from concrete to steel fiber, which restrained the further extension of cracks. The second reason is that the expansion of MgO was restrained by steel fiber, which resulted in self-stress. In the free state, the expansion energy produced by the hydration of MgO made the concrete expand outwards. However, under the constraint of steel fiber, the expansion energy was used to tension the fiber, resulting in self-stress. In this way, the tensile strength of concrete is obviously improved.

3.3. Combined Action of Steel Fiber and MgO on Chloride Diffusion Resistance

Table 3 shows the chloride diffusion coefficients (CDC) of concrete at 3, 7, 28, 60, and 180 days. As can be seen from Figure 8, with the increase in curing time, CDC of concrete with different mixing ratios gradually decreased, and the descending rate in the early stage (0–28 days) was relatively larger, while the rate in the later stage gradually decreased. The results showed that early curing of concrete had a significant effect on CDC. As the curing age increased, the cementitious materials continued to hydrate, the densification of concrete became higher, and the pores in concrete were gradually reduced, which was beneficial to reduce CDC and improve the durability. No matter how we changed the amount of fiber and MgO, the test results always obeyed this rule. Therefore, we suggest that in constructions, it is necessary to increase the time of early maintenance of concrete to improve the durability.

Type	euring rige							
Type	3 d	7 d	28 d	60 d	180 d			
Ref (RC)	15.6	15.4	10.8	8.8	6.8			
8%MgO (MC)	15.4	15.2	10.7	8.4	6.7			
0.5%Fiber (SFRC)	14.9	14.5	9.8	8.0	6.2			
8%MgO + 0.5%Fiber (SFRMC-1)	14.8	14.0	8.6	7.7	5.1			
8%MgO + 1%Fiber (SFRMC-2)	13.7	11.2	6.4	6.2	4.8			
8%MgO + 1.5%Fiber (SFRMC-3)	11.1	10.3	7.2	6.2	4.9			



Figure 8. Changes of chloride diffusion coefficient.

Figure 8 shows that using MgO separately had little effect on CDC. At 180 days, CDC of MC was only 1.2% lower than that of RC. Different from MgO, steel fiber had both positive and negative effects on the durability of concrete. The first was the positive effect. Steel fiber could effectively inhibit the generation of cracks and reduce CDC. The other was the negative effect. The addition of steel fiber increased the number of the interface between fiber and matrix and provided a channel for the transfer of chloride ions, which possibly improved CDC. It can be found from Table 3, when the steel fiber was less than 1%, the positive factor played a major role. CDC decreased with the increase of steel fiber. Compared to RC, CDC of SFRC, SFRMC-1, SFRMC-2 decreased by 5.5%, 8.8%, and 36.7%, respectively at 7 days. However, when the steel fiber continued to increase, the negative effect gradually dominated, and CDC of SFRMC-3 was larger than that of SFRMC-2, which indicated that when steel fiber was too

large, the negative effect caused by steel fiber was larger than the positive effect. Therefore, for SFRMC, the optimal content of steel fiber should not be more than 1%. In the later stage of curing (180 days), owing to the combined action of MgO and steel fiber, CDC of SFRMC-2 was 4.8×10^{-9} cm²/s, which was only 70.8% of RC, and the durability of concrete was obviously enhanced.

3.4. Combined Action of Steel Fiber and MgO on Porosity of Concrete

As an important part of concrete, the pore structure directly affected the mechanical properties and durability of concrete [24–26]. Therefore, the study on the effect of MgO and steel fiber on concrete is inseparable from the research on the pore structure. The pores can be classified into four categories: cementitious pores (<10 nm), transitional pores (10–100 nm), capillary pores (100–1000 nm), and macro pores (>1000 nm) [27]. Figure 9 shows the pore diameter distribution curve of concrete. Figure 10 shows the statistics of pore size.



Figure 9. Porosity distribution curve of concrete.



Figure 10. Statistics of concrete pore size.

Figures 9 and 10 show that compared to RC, the porosity of MC increased by 5.7%, indicating that the expansion of MgO is harmful to pore structure. The porosity of SFRC decreased by 1.5% compared to RC, and the number of large pores decreased by 15.9%, indicating that the steel fiber optimized the pore structure of concrete.

At the same time, when MgO and steel fiber were used together, the porosity continued to decrease. Compared to RC, the porosity of SFRMC-1, SFRMC-2, SFRMC-3 decreased by 6.2%, 18.4%, and 20.6%, respectively. In addition, the number of large pores was significantly reduced, cementitious and excessive pores were increased, and the connectivity of pores was reduced, which was also the main reason for the improvement of the chloride diffusion resistance of SFRMC.

3.5. Combined Action of Steel Fiber and MgO on Interfacial Transition Zone

To further analyze the principle of performance increase of SFRMC, the microstructure of concrete, especially the interfacial transition zone (ITZ) between fiber and matrix, was explored. The microstructure of ITZ with different mix ratios is shown in Figure 11. Figure 11 shows that in SFRC, there were obvious long and wide gaps in ITZ due to the shrinkage of matrix. Besides, tensile stress was produced because steel fiber restricted the shrinkage of matrix. In addition, when the tensile stress was larger than the tensile strength of concrete, the vertical crack was produced, leading to a reduction in durability.

Different from SFRC, when steel fiber and MgO were used at the same time, the wide gap at the interface disappeared. This change was mainly due to the extrusion and filling effect of MgO. Owing to the expansion under the constraint of steel fiber, self-stress was generated in ITZ (Figure 12). The self-stress made the biting force and friction force greatly increased, made ITZ become dense, and the boundary become fuzzy.



Figure 11. Interfacial transition zone (ITZ) between the steel fiber and matrix. (a) SFRC, (b) SFRMC-2.



Figure 12. Self-stress caused by expansion extrusion at the interface.

4. Conclusions

Considering the defects of MgO and steel fiber when they were used separately, MgO and steel fiber were used at the same time in this paper. The influence of combined action on split tensile strength, chloride diffusion resistance, and pore structure of concrete was discussed in detail. Through detailed experimental research and theoretical analysis, the following conclusions could be drawn:

(1) For concrete, MgO and steel fiber could be used at the same time. In SFRMC, the steel fiber played the role of bearing and limiting expansion at the same time. The combination of MgO and steel fiber improved the split tensile strength and chloride diffusion resistance of concrete significantly. Compared to RC, the tensile strength of SFRMC-1, SFRMC-2, and SFRMC-3 increased by 3.1%, 61.3%, and 64.5%, and CDC decreased by 8.8%, 36.7%, and 33.1%, respectively.

(2) When steel fiber and MgO are used simultaneously, the two have a synergistic effect and the performance of the concrete is greatly improved. The combined action of steel fiber and MgO reduced the porosity of concrete, which could not be achieved by steel fiber and MgO separately. Compared to RC, the porosity of SFRMC-1, SFRMC-2, and SFRMC decreased by 6.2%, 18.4%, and 20.6%, respectively.

(3) Through the analysis of pore structure, the mechanism of performance enhancement of SFRMC was studied. Owing to the expansion of MgO was restrained by steel fiber, MgO changed from outward expansion to inward extrusion, resulting in filling and compaction effect. At the same time, self-stress was produced in ITZ, which improved the interfacial strength between steel fiber and matrix.

Author Contributions: Conceptualization, F.J. and M.D.; methodology, L.M.; software, F.J.; validation, F.J., M.D., L.M. and W.W.; formal analysis, M.D. and L.M.; investigation, F.J., M.D., L.M. and W.W.; resources, F.J. and M.D.; data curation, W.W.; writing—original draft preparation, F.J. and M.D.; writing—review and editing, F.J., M.D., L.M. and W.W.; visualization, F.J.; supervision, F.J., M.D., L.M. and W.W.; project administration, F.J.; funding acquisition, F.J. All authors have read and agreed to the published version of the manuscript.

Funding: This research was funded by Science and Technology Development Plan of Suzhou (SNG201904), National Key Research and Development Plan of China (2017YFB0309903-01) and Transportation Science and Technology Planning Project of Shandong Province (2018B37-02).

Acknowledgments: The authors would like to thank Zhongyang Mao from Nanjing Tech University for his precious contribution in the experiments.

Conflicts of Interest: The authors declare no conflict of interest.

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